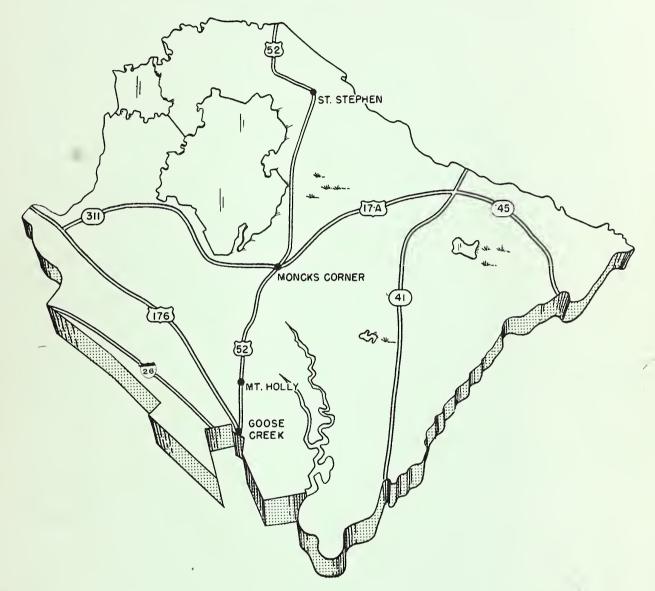
Historic, Archive Document

Do not assume content reflects current scientific knowledge, policies, or practices.



aTC175 .U53

FEASIBILITY STUDY OF REQUIREMENTS FOR MAIN DRAINAGE CANALS



BERKELEY COUNTY, SOUTH CAROLINA

Prepared under sponsorship of BERKELEY COUNTY

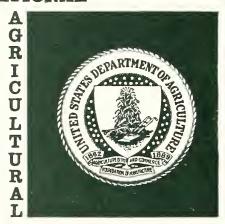
and

BERKELEY SOIL CONSERVATION DISTRICT

in cooperation with the

U. S. Department of Agriculture Soil Conservation Service U. S. Forest Service 473-33 Bookplate (1-64)

NATIONAL



LIBRARY

Foreword

The interrelationship of man, water, and land has always been an important factor in the development and growth of any community. In Berkeley County, South Carolina, the absence of a well defined drainage pattern has hampered the proper development of this relationship.

The feasibility study of requirements for main drainage canals in Berkeley County is the outgrowth of interest on the part of the county authorities and the Berkeley County Soil Conservation District Supervisors who, through their foresight, saw the need of such a plan in order to enhance the potential development of the county. This plan, as developed, is a direct result of such foresight. It is the first step toward solving the drainage needs of the county, which is recognized by all concerned as a problem of first priority. Agencies at all levels of government - local, county, state, federal - as well as private enterprise and numerous individuals, cooperated in the development of the plan. The Berkeley County delegation contributed largely to the cost of the project, including the publication of this report; technical assistance was furnished by the Soil Conservation Service.

The plan will provide a firm basis for action by county officials in determining needed legislation and methods of financing the necessary drainage improvements as well as establishing priorities of work. The cooperation of other agencies, groups, and individuals in the use of the plan also will be encouraged.

U.S. DEPT. IF MARICULTURAL ATIONAL AGRICULTURAL LIBRARY

SEP 1 - 1990

CATALOGING = PREP.

ı

CONTENTS

	Page		Page
Introduction and Scope	1	Factors Considered in Preparation	
Factors Affecting Drainage	1	of Plan	16
Topography	1	Engineering Considerations	17
Tidal Ranges	1	Design	
			17
Rainfall	1	Acquisitions of Rights-of-way	17
Soils	3	Maintahance of Channels	17
Culverts	3	Obstructions	17
Urbanization	3	Definition of Terms	17
Existing Drainage System	4	Technical References	18b
Maintenance	4	Authority and Acknowledgement	18b
Drainage Principles	4	Explanation of Engineering Data Tables -	19
Surface Drainage	4	Engineering and Design Data	
			21
Sub-surface Drainage	4	Area 1 - Hanahan - Goose Creek -	
The Drainage System	4	Carnes Gressroads - Oaklėy+-	21
The Collection System	4	Area 2 - Moneks Corner - Oakley	25
The Disposal System	4	Area 3 - Washamussaw - Cooper's	
The Outlet	4	Store - New Hope - Lebanon +	28
Drainage Requirements	4	Area 4 - San a Ridge - Pringletown	34
Design Criteria	5	Area 5 - Crost	37
Drainage Coefficients	5	Area 6 - Eamy town - Fineville -	
Velocity	5	St. Stephen	40
Channel Cross Section	5	Area 7 - St. Stephen - Alvin -	
Values of Roughness Coefficient "n" -	5	Jamestown	45
9	7	Area 8 - Russellville - Bonneau -	40
Channel Depth and Width			0 10
Side Slopes	7	Macedonia	49
Design at Culverts	7	Area 9 - Macedonia - Bethera - Gough -	54
Right-of-way Requirement - Berm Width,		Area 10 - Childsbury - Cördesville -	
Spoil Bank	8	Witherbee	58
Dikes, Conduits and Pumps	8	Area 11 - Jamestown - Shulerville -	
Needs and Location	8	Honey Hill	63
			00
Design Criteria	8	Area 12 - Bethera - Huger - Eccles	- 5
Description of Areas	10	Church - Green Bay	66
Area 1 - Hanahan - Goose Creek -		Area 13 - Pompion Chapel - Charity	
Carnes Crossroads - Oakley	10	Church - Wando - Daniel's Island	71
		Daniel B 151and	7 4
· · · · · · · · · · · · · · · · · · ·			
Area 2 - Moncks Corner - Oakley		General Soil Map Follows Page	
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store -	10	General Soil Map Follows Page Maps Showing the Drainage	75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon	10	General Soil Map Follows Page Maps Showing the Drainage	
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown	10	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page	75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon	10	General Soil Map Follows Page Maps Showing the Drainage	75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown	10 10 10	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures	75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville -	10 10 10 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient	75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen	10 10 10	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves	75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin -	10 10 10 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient	75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown	10 10 10 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves	75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau -	10 10 10 11 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves	75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia	10 10 10 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width	75 75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough -	10 10 10 11 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and	75 75 6
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough -	10 10 10 11 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure	75 75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia	10 10 10 11 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing	75 75 6
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee	10 10 10 11 11 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike - R/C	75 75 6
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee Area 11 - Jamestown - Shulerville -	10 10 10 11 11 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike - R/C Pump Structure Tide Gate and	75 75 6 8
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee Area 11 - Jamestown - Shulerville - Honey Hill	10 10 10 11 11 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike + R/C Pump Structure Tide Gate and Channels	75 75 6
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee Area 11 - Jamestown - Shulerville - Honey Hill Area 12 - Bethera - Huger - Eccles	10 10 10 11 11 11 11 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike + R/C Pump Structure Tide Gate and Channels Figure No. 5 - Index to	75 75 6 8 9
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee Area 11 - Jamestown - Shulerville - Honey Hill Area 12 - Bethera - Huger - Eccles Church - Green Bay	10 10 10 11 11 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike + R/C Pump Structure Tide Gate and Channels Figure No. 5 - Index to	75 75 6 8 9
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee Area 11 - Jamestown - Shulerville - Honey Hill Area 12 - Bethera - Huger - Eccles	10 10 10 11 11 11 11 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike - R/C Pump Structure Tide Gate and Channels Figure No. 5 - Index to Map Sheats	75 75 6 8 9
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee Area 11 - Jamestown - Shulerville - Honey Hill Area 12 - Bethera - Huger - Eccles Church - Green Bay	10 10 10 11 11 11 11 11	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike - R/C Pump Structure Tide Gate and Channels Figure No. 5 - Index to Map Sheets	75 75 6 8 9
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee Area 11 - Jamestown - Shulerville - Honey Hill Area 12 - Bethera - Huger - Eccles Church - Green Bay Area 13 - Pompion Chapel - Charity Church - Wando - Daniel's Island	10 10 10 11 11 11 11 11 12 12	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike + R/C Pump Structure Tide Gate and Channels Figure No. 5 - Index to	75 75 6 8 9
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee Area 11 - Jamestown - Shulerville - Honey Hill Area 12 - Bethera - Huger - Eccles Church - Green Bay Area 13 - Pompion Chapel - Charity Church - Wando - Daniel's Island Soil Associations	10 10 10 11 11 11 11 12 12 12	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Ditc + R/C Pump Structure Tide Gate and Channels Figure No. 5 - Index to Map Chests	75 75 6 8 9
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee Area 11 - Jamestown - Shulerville - Honey Hill Area 12 - Bethera - Huger - Eccles Church - Green Bay Area 13 - Pompion Chapel - Charity Church - Wando - Daniel's Island - Soil Associations	10 10 10 11 11 11 11 12 12 12	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike - R/C Pump Structure Tide Gate and Channels	75 75 6 8 9 75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee Area 11 - Jamestown - Shulerville - Honey Hill Area 12 - Bethera - Huger - Eccles Church - Green Bay Area 13 - Pompion Chapel - Charity Church - Wando - Daniel's Island - Description of Soil Associations and Their Drainage Problems	10 10 10 11 11 11 11 12 12 12 12	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike - R/C Pump Structure Tide Gate and Channels Figure No. 5 - Index to Map Chests	75 75 6 8 9
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown	10 10 10 11 11 11 11 12 12 12 12 13	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike - R/C Pump Structure Tide Gate and Channels	75 75 6 8 9 75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen	10 10 10 11 11 11 11 12 12 12 12	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike - R/C Pump Structure Tide Gate and Channels	75 75 6 8 9 75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown	10 10 10 11 11 11 11 12 12 12 13 13 13	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike - R/C Pump Structure Tide Gate and Channels Figure No. 5 - Index to Map Sheets	75 75 6 8 9 75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen	10 10 10 11 11 11 11 12 12 12 13 13 13	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike - R/C Pump Structure Tide Gate and Channels	75 75 6 8 9 75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown	10 10 10 11 11 11 11 12 12 12 13 13 13	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike - R/C Pump Structure Tide Gate and Channels Figure No. 5 - Index to Map Sheets	75 75 6 8 9 75 2
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia - Bethera - Gough - Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee	10 10 10 11 11 11 11 12 12 12 13 13 13 14	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike + R/C Pump Structure Tide Gate and Channels	75 75 6 8 9 75
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee	10 10 10 11 11 11 11 12 12 12 13 13 14	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike + R/C Pump Structure Tide Gate and Channels	75 75 6 8 9 75 2
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia Area 9 - Macedonia - Bethera - Gough - Area 10 - Childsbury - Cordesville - Witherbee	10 10 10 11 11 11 11 12 12 12 13 13 14 14	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike + R/C Pump Structure Tide Gate and Channels	75 75 6 8 9 75 2
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown	10 10 10 11 11 11 11 12 12 12 12 13 13 14 14 11 15	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike + R/C Pump Structure Tide Gate and Channels	75 75 6 8 9 75 2
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia	10 10 10 11 11 11 11 12 12 12 13 13 14 11 15 15	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike + R/C Pump Structure Tide Gate and Channels	75 75 6 8 9 75 2
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown	10 10 10 11 11 11 11 12 12 12 12 13 13 14 14 11 15	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike + R/C Pump Structure Tide Gate and Channels	75 75 6 8 9 75 2
Area 2 - Moncks Corner - Oakley Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon Area 4 - Sand Ridge - Pringletown Area 5 - Cross Area 6 - Eadytown - Pineville - St. Stephen Area 7 - St. Stephen - Alvin - Jamestown Area 8 - Russellville - Bonneau - Macedonia	10 10 10 11 11 11 11 12 12 12 13 13 14 11 15 15	General Soil Map Follows Page Maps Showing the Drainage Plan Follow Page Figures Figure No. 1 - Drainage Coefficient Curves Figure No. 2 - Typical Main Ditch Cross-Section Showing Basis for Determining Right-of-way Width Figure No. 3 - Typical Profile and Cross-Section and Pump Structure Figure No. 4 - Plan View Showing Typical Installation of Dike + R/C Pump Structure Tide Gate and Channels	75 75 6 8 9 75 2

FEASIBILITY STUDY OF REQUIREMENTS FOR MAIN DRAINAGE CANALS BERKELEY COUNTY, SOUTH CAROLINA

Introduction and Scope

The Feasibility Study of Requirements for Main Drainage Canals in Berkeley County is the logical first step toward solving the excess water problem. The purpose of the study is to point out the extent and severity of the drainage problem in the county and to furnish a guide to determine the physical feasibility and the estimated cost of the needed improvements. To accomplish this purpose, a system of main drainage canals has been developed for the major watersheds of the county and a discussion of some of the principal criteria used in design given.

The data in this report are based on reconnaissance surveys, information presently available, and on knowledge gained by long experience in planning and establishing drainage facilities in the county. The data are adequate for the purpose of determining preliminary design and cost estimates but are not adequate for the preparation of final construction plans, designs, and costs. The data herein presented, however, can be used by qualified engineers as guides in securing detailed information for these purposes. Included also are technical references which can supply information for the final engineering investigations, plans, and designs.

The use of most of the land in Berkeley County is dependent on adequate drainage. The lack of drainage is the principal detriment to the development of the land resources of the county. It results in frequent and costly crop damage on agricultural land and to property damage and



SC-2033-1

AGREEMENT SIGNED—Senator Rembert C. Dennis signs an agreement for the publication of the Feasibility Study of Requirements for Main Drainage Canals in Berkeley County. Looking on, left to right are: H. H. Harvey, Ir., Chairman, Board of Soil Conservation District Supervisors; H. N. West, Member of Berkeley County Delegation; Clyde D. Umphlett, Berkeley County Supervisor; Henderson Guerry, Member of Berkeley County Delegation and Dr. T. S. Buie, State Conservationist, Soil Conservation Service, who also signed the agreement.

Photo by Maxie Roberts The State-Record Company, Columbia, S.C. disruption of facilities, both public and private, in urban and industrial areas.

The need to reduce flooding through improvement of drainage is recognized as a problem of first priority.

Factors Affecting Drainage

Berkeley County is located in the southeastern part of South Carolina near the Atlantic Seaboard, The southern part of the county is about 5 miles from the Atlantic Ocean. The physical features of the county, including topography, tidal ranges, rainfall, soils, and land use changes, result in complex drainage problems. All of these are inter-related. A brief discussion of how the physical features affect drainage follows.

Topography

Topography is a severely limiting factor affecting drainage. The land is generally level with slight undulations. Sharp breaks in topography occur along tidal streams and marshes. Elevations in the county range from mean sea level to 105 feet above mean sea level, with most of the drainage problems occurring between the 5- to 40-foot contour. The southern part of the county outlets into tidal creeks, which are subject to tidal fluctuations. The natural interior drains in most cases outlet into these streams at a higher elevation, which minimizes the effect of tidal changes. The natural drains are broad, have flat grades, and are heavily vegetated. In their natural state, little or no channel exists, causing extreme ponding in depressed areas.

Tidal Ranges

The tidal effects along the rivers in the southern part of the county are very complex, and highly variable, dependent on the force, direction, and duration of winds and other weather events occurring seaward. Predicted or normal range of tides above mean low water, with no consideration of wind effects, is 5.2 feet, with spring tides ranging to 6.8 feet. However, daily tide records maintained by the U. S. Weather Bureau, Charleston, S. C., show that there is considerable variation between the predicted and actual tide ranges due to wind. Generally, tide heights have a departure of 1.0 - 1.5 feet below normal. Storm tides which occur when sustained winds along the coast exceed 40 miles per hour have a departure from normal of 2.5 to 3.0 feet above normal. A thorough knowledge of tidal action is essential in proper planning and design of drainage systems and supporting structures.

Rainfall

U. S. Weather Bureau Records, Table No. 1, shows monthly and yearly rainfall records for Pinopolis Dam, South Carolina Public Service Authority's Hydro-Electric Plant near Moncks Corner, South Carolina. The average yearly

TABLE NO.. I RAINFALL DATA - U. S. WEATHER BUREAU PINOPOLIS DAM, S.C.

TOTAL PRECIPITATION

Year	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	Oct.	Nov.	Dec.	Annual
1931	3.55	0.80	3.82	2.25	8.47	4.18	3.97	1.97	0.25	0.20	0.09	4.02	33.57
1932	2.33	3.35	1.56	1.27	6.18	12.25	0.95		4 .89	5.93		1.20	
1933	3.34	5.64	1.63	2.47	1.62	4.79			1.36	1.58	1.75	0.00	
1934	0.93	3.99	1.67	1.73	1.57	2.20		7.95	3.58	2.52	2.49	1.20	
1935	1.26	1.69	1.78	2.54	5.11	2.23	8.56	5.50	8.06	1.95	1.90	2.96	43.54
1936	3.96	4.62		4.32	1.10	4.39	6.78	4.03	1.95	10.64	0.46	5.09	
1937	4.70	4.49	2.70	5. 85	1.05	5.23	5.80	7.61	4 .83	2.22	3.90	2.66	51.04
1938	0.80	0.81	0.50	5.56	3.52	3.10	3.70	2.01	5.52	1.40	0.88	1.99	29.79
1939	2.84	8.79	3.69	2.60	5.10	5.62	2.79	6.88	1.24	0.68	0.32	2.00	42.55
1940	4.43	6.00	3.50	1.71	5.64	4.36	4.09	12.70	2.05	0.15	1.59	2.87	49.09
1941	1.05	1.76	5.31	3.10	T*	9.81	8.63	3.96	1.04	0.18	1.26	7.16	43.26
1942	1.40		6.37	1.68	2.23	6.46	3.49		2.82	0.11		2.95	
1943	2.84	0.98	6.74	2.92	2.17		6.38	6.36	2.49	0.01	1.50	3.32	
1944	3.41	5.22	9.34	3.07	1.60	1.31	2.41	1.64	4.24	6.51	1.39	0.85	40.99
1945	1.52	3.53	1.47	1.98	2.89	5.75	10.98	6.91	17.36	4.02	1.42	5.57	63. 40
1946	3.69	1.96	4.13	3.04	5.39	1.63	6.94	3.44	1.93	3.73	2.32	0.50	38.70
1947	3.87	0.18	6.31	4.37	6.32	3.75	8.00	6.07	7.23	2.67	4.94	4.73	58.44
1948	3.48	5.62	7.60	3.33	4.95	1.32	6.77	5.54	6.03	2.49	7.70	3.90	58.73
1949	0.72	6.09	2.18	2.96	2.75	5.35	3.87	10.40	4.54	1.01	1.85	1.04	42.76
1950	0.48	0.81	4.38	2.00	4.01	3.16	7.52	6.56	7.67	3 .25	1.23	4.64	45.71
1951	1.31	1.17	3.39	1.62	0.89	6.58	6.88	3.65	1.76	0.65	2.95	2.21	33.06
1952	1.10	5.22	5.67	2.72	3.95	2,24	6.91	10.65	8.21	1.95	2.08	2.35	53.05
1953	0.87	6.00	4.63	0.82	3.08	4.33	5.27	8.26	5.37	1.45	2.13	7.06	49.27
1954	1.40	1.13	1.70	3.52	3.25	0.57	1.71	2.77	1.45	3.03	1.11	2.50	24.14
1955	3.77	1.28	1.21	3.08	6.67	5.90	5.62	11.44	7.91	1.05	2.60	1.74	52.27
1956	1.77	4.69	3.17	2.10	3.21	2.95	4.16	7.81	4.15	4.09	0.54	0.98	39.62
1957	1.66	2.03	5.09	0.95	5.98	4 .62	4.04	5.50	7.26	1.13	3.95	2.60	44.81
1958	5.21	3.78	5.64	9.40	5.38	13.65	8.26	6.23	1.16	5.34	0 .50	3.24	67.79
1959	3.75	4.88	7.66	2.30	2.62	2.01	14.41	3.53	8.75	8.69	1.26	2.74	62.60
1960	4.00	4.61	3.74	2.53	1.85	6.80	10 88	3.72	7 33	1.23	0.73	2.05	49.47
1961	2.23	4.55	2 .64	8.36	4.43	3.42	4.82	8.43	3.33	1.39	1.50	2.11	47.21
1962	4.87	2.58	6.61	3.81	1.09	6.50	5.04	6.93	5.91	1.58	2.68	1.61	49.21
Average					2								
Rainfall	2. 58	3.49	4.06	3.10	3.68	4.72	5.99	6. 15	4.74	2.59	1.97	2.81	46.70
	1				L			<u> </u>			L	L	

In the above table information shown from January 1931 to June 1943 is taken from Pinopolis weather station. From July 1943 to December 1962 information shown is taken from the weather station at Pinopolis Dam.

- T^* means Trace not enough to measure.
- -- No record available.

PRECIPITATION EXTREMES

	Maximum Monthly	Year	Minimum Monthly	Year
January	5.21	1958	0.48	1950
February	8.79	1939	0.18	1947
March	9.34	1944	0.50	1938
April	9.40	1958	0.82	1953
May	8.47	1931	T*	1941
June	13.65	1958	0.57	1954
July	14.41	1959	0.95	1932
August	12.70	1940	1.64	1944
September	17.36	1945	0.25	1931
October	10.64	1936	0.01	1943
November	7.70	1948	0.09	1931
December	7.16	1941	0.00	1933

T* means Trace - Not enough to measure.

RAINFALL IN INCHES FOR SELECTED DURATIONS!*

	30 Min.	1 Hour	2 Hours	3 Hours	6 Hours	12 Hours
1 Year	1.4	1.8	2.1	2.3	2.6	3. 1
2 Years	1.6	2.0	2.4	2.7	3.1	3.6
5 Years	2.0	2.5	3.1	3.4	4.1	4.8
10 Years	2.3	2.9	3.5	3.9	4.6	6.2
25 Years	2.5	3.3	4.2	4.5	5.7	6.5
50 Years	2.9	3.7	4.5	5.1	6.2	7.2
100 Years	3 .2	3.9	5.1	5.6	6.7	8.2

^{*} U. S. Weather Bureau Technical Paper No. 40 - "Rainfall Frequency Atlas of the United States"

rainfall of 46.70 inches would not cause a serious drainage problem if it were evenly distributed. The most serious drainage problem in areas along the tidal creeks is created by the high intensity, short duration rain storms occurring during periods of high tides and prevailing easterly winds. Also, Lake Moultrie, in the northern part of the county, creates a drainage problem due to its elevation and the topography of the surrounding areas. The design of drainage systems and supporting structures is related to the amount of runoff that can be expected from storms of differing intensities and durations.



ROAD FLOODED—Heavy rains flooded section of county road in intensive farming area near St. Stephen, S.C. Adequate outlets would have carried off excess water.



 $\it HOME$ AND LAND FLOODED—Causing crop and property damage in Russellville section of Berkeley County.

Soils

A description of soil associations in Berkeley County is contained on pages 13 to 16. Soils have characteristics which decidedly influence the need for, and the degree of, drainage. Some of the more important characteristics are: depth, infiltration, permeability, texture, structure, water-holding capacity, water-table depth, and slope. A knowledge of these characteristics, as well as of the engineering properties of soils, is essential in planning, designing and constructing an adequate drainage system. Fine (clayey) textured soils have little or no subsurface water movement and can be drained only by removal of surface water by means of shallow

surface ditches. Sandy soils, having high water tables or fluctuating water tables, respond to sub-surface drainage, but present problems in the design of open ditches. These problems include: (a) side slope sloughing, which limits depth of cuts; (b) limitation of the velocity of flow; and (c) sedimentation.

Culverts

Culverts for road and railroad drainage generally lack capacity to handle runoff from high intensity storms and are frequently installed with invert elevations too high. They are a serious bottleneck to the rapid disposal of runoff and cause local flooding. The problem is less severe on primary roads than on secondary roads. Culverts are almost universally inadequate on unpaved and farm roads.

Drainage structures in driveways paralleling streets and roads in established subdivisions and towns are critical factors in contributing to poor local drainage. Head losses alone resulting from the widespread use of under-designed culverts in residential areas create local flooding problems.



Two culverts on right were installed too high to provide adequate drainage. Culvert on left was added when new drainage canal was constructed.

Urbanization

Urbanization of areas around Moncks Corner, Goose Creek, and Hanahan is having an adverse effect on drainage. Some of the drainage facilities now in use were established to handle the agricultural needs of the area. They are not adequate to handle runoff resulting from urbanization. Roof tops, paved roads, compaction, raised water tables resulting from septic tanks and tile field installations, grading and elimination of some ditches during urban development, have created conditions approaching 100 percent runoff. As urbanization continues, the present drainage facilities will become increasingly inadequate to handle runoff.

There is a need for regulations to insure that adequate drainage canals and drainage structures are installed as these areas develop.



Flooded streets and homes in subdivision following heavy rains. Adequate drains are needed.

Existing Drainage System

With the exception of some recently excavated canals, drainage systems in rural and urban areas are generally inadequate in depth and capacity, and have very flat grades. An important additional factor contributing to this problem is the lack of legal authority to secure adequate rights-of-way for proper ditch design, spoil management, and access for maintenance. Rights-of-way in the past were usually limited to the width which the landowner was willing to donate, which in most cases was less than thirty feet.

Existing flat grades are the result of discharging canals - (1) into tidal marshes at mean sea level elevation rather than at mean low water elevation, or (2) discharging into swamps which are not adequate outlets in their present state since they generally pond water for long periods of time following heavy rainfall.

Existing canals are usually located in natural water courses. However, in many instances alignment is poor, since attempts were made to accomodate the canals to existing property lines or other physical features inconsistent with good channel flow conditions.

Maintenance

Lack of adequate maintenance is a factor which affects the capacity of canals. The existing drainage canals in most of the county were dug by hand many years ago; some of them were enlarged by the Works Progress Administration in the 1930's. They have nearly vertical side slopes, with spoil placed immediately next to the ditch. Practically all canals have high spoil banks which are covered by heavy growth of trees and brush, making access very difficult. Continuous spoil banks for long distances prevent surface drainage from adjacent areas and result in ponding. The extent of machine maintenance is limited at present, due to these conditions and also to the lack of legal easements permitting access.

Drainage Principles

The purpose of this report is to present a plan for the location and needed capacities of main drainage canals. This is, however, only the first step in the establishment of a complete

drainage system. Drainage systems are divided into two broad categories - surface drainage, and sub-surface drainage.

Surface Drainage

Surface drainage removes excess water, by gravity, from the land surface to an outlet. Surface water can best be moved by shallow channels or by grading the land surface to a uniform slope primarily on cultivated land. To insure water movement along the surface to an outlet without ponding is a very important function of the drainage system. Surface drainage facilities are particularly applicable to soils having slow permeability rates, to the drainage of low pockets to prevent ponding, and to the diversion of water from protected areas. Also, they collect and convey water to natural channels or to constructed channels.

Sub-surface Drainage

Sub-surface drainage removes water from beneath the surface of the soil by facilities which create a difference in hydraulic head. The resulting hydraulic head causes water to move through the soil to an outlet. Sub-surface drainage may be accomplished by open ditch drains or by tile drains. Open ditch drains have an added advantage because they can also collect and remove surface water. Tile drains, with certain precautions, can remove surface water by simulating a small storm sewer system.

The purpose of sub-surface drainage is to lower the water table to a point where it will not interfere with plant growth or the use of the land for residential or other purposes. The minimum depth below the surface at which water tables should be maintained depends on the use of the land. Water tables, fluctuating upwards to or near the surface, may not be as great a problem in agricultural areas as they would be in populated areas.

The Drainage System

The components of a Drainage System are as follows:

The Collection System - is that part of the drainage system which first picks up water from the land. It may consist of shallow trapezoidal ditches, having flat side slopes; V or W type ditches, bedding, or grading the land surface in urban areas. This is a part of the drainage system which cannot be neglected if the system is to perform adequately.

The Disposal System - receives water from the collection system and conveys it, usually in an open channel, to the outlet. Generally, this report concerns itself with this part of the drainage system.

The Outlet - is the end point of any segment of a drainage system beyond which the ditch, storm sewer, or the system no longer guides or controls the water it discharges.

Drainage Requirements

The drainage system should be designed to prevent flooding in critical parts of the watershed for a period of time sufficient to cause

damage or disrupt utilities and services. For urban areas, design should provide for the removal of runoff from the design storm with a minumum of flooding. In agricultural areas, the degree of protection required by crops varies considerably, depending on their tolerance to the amount and duration of excess water. Truck crops are the most susceptible to damage from excess surface water, some of which occurs when they are flooded for the relatively short period of 24 hours or less. General crops such as corn and grain are not as susceptible to damage as truck crops, and pasture is still less susceptible to water damage. Woodland areas are the least subject to damage from flooding for prolonged periods.

Poorly drained soils adversely affect the use of the land for most purposes. On agricultural land, high water tables restrict root penetration; soil temperature is lowered, air circulation is severely limited, dependent on the degree of soil saturation. Wet spots in the field delay farm operations and shorten the growing period.

Poorly drained soils in residential areas, in addition to their effects on ornamental plants and lawns, adversely affect the construction, maintenance, and use of roads and streets. They also limit or prohibit the development of some areas, preventing the proper functioning of septic tank tile fields, and contribute to health hazards.



Flooded corn field caused by lack of proper drainage.



FLOODED FIELD--Water standing on field like this makes cultivation difficult.

Design Criteria

The design of drainage systems and supporting structures is based on Hydrology and Hydraulics and this report will limit itself to the application of these sciences as they apply to the solution of such problems. References for more detailed information on design of open channels, closed conduits, culverts, dikes, pumps, tide gates, and other engineering structures ultimately involved in establishing a drainage system are listed on pages 21 - 75.

Drainage Coefficients

The drainage coefficient is the rate of removal of runoff to provide a specific degree of drainage protection to an area. Land use, soils, topography, and rainfall intensities and duration determine the selection of drainage coefficients. A series of four curves have been developed from which required drainage capacities of open ditches can be computed, dependent on the land use. (See Figure No. 1) The highest curve is for urban use followed in descending order for truck crops, general crops, and woodland.

The use of these curves provides for the removal, in 24 hours time, of the following amounts of runoff:

Urban curve - 4.39 inches Truck crops - 3.33 inches General crops - 1.67 inches Woodland - 0.37 inches

The curve for urban areas reflects a peak runoff for a 10-year frequency.

Velocity

Soil characteristics, the shape of the channel, and available means for stabilization of the soil after construction, determine the maximum safe velocity. The optimum velocity for channels, based on soil conditions in Berkeley County, is approximately 2 feet per second. The soils are predominately fine sands. Sedimentation occurs when velocities are less than $1\frac{1}{2}$ feet per second which is frequently caused by vegetative growth. Erosion will occur in most soils at velocities in excess of 3 feet per second. Design of channels in the fine, water bearing sands must consider the need for checking erosion and bank caving that will occur immediately following construction when water tables are high.

Velocities should be designed after a thorough investigation of soil conditions to the depth of proposed channels.

Channel Cross Section

Values of Roughness Coefficient "n"

All channel cross sections were computed by use of Manning's formula for determining velocities. This is:

 $V = \frac{1.486}{n} \times r^{2/3} \times s^{1/2}$

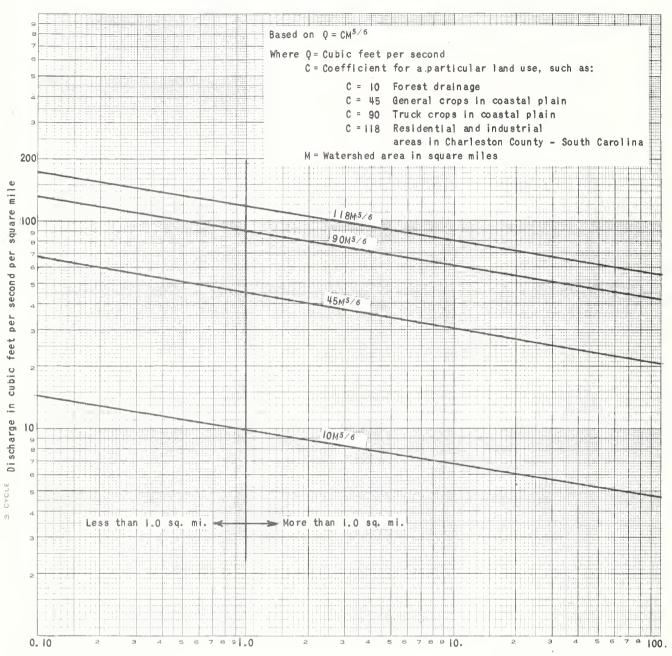
where: n = Roughness coefficient

r = Hydraulic radius

s = Slope in feet per foot
 along the ditch

The proper design of a ditch cross section required the selection of the proper value of "n". Side slopes of the ditch as well as depth and allowable velocities are fixed primarily by soil conditions and proposed maintenance methods.

Figure No. I - Drainage Coefficient Curves



Watershed area in square miles

The following tabulations were used for selection of values of "n" for Manning's formula in the design of main canals with good alignment:

ydraulic Radius*	<u>''n''</u>
Less than 2.5	.045
2.5 to 4.0	.040
4.0 to 5.0	.035
over 5.0	.030

* The hydraulic radius is obtained by dividing the proposed area of the channel cross section by its wetted perimeter.

In newly dug channels, roughness is lower and velocities higher. A realistic roughness coefficient was selected anticipating flow retardance features, such as vegetative growth and sediment several years after construction. Where the design velocity was near an erosive value, corrective measures were planned.



These two draglines are in process of construction of a major drainage canal. This picture shows the need for adequate cleared right-of-way in which to dig the required canal and, also, provide sufficient space for the excavated soil.

Channel Depth and Width

Depth of channel is an important design consideration. The channel must be deep enough to tap and provide for the escape of ground water,

and to provide for the safe entrance of the longer lateral ditches and tile drains. Other considerations favoring a deeper channel with a resulting narrower bottom width are: less right-of-way is required, vegetative growth on the wetted perimeter is reduced, and conditions are less favorable for the formation of sandbars. A channel roughly as deep as its bottom width - within economic limits - will remain effective for a longer period because it has most favorable hydraulic characteristics.

A minimum bottom width of 3.0 feet was designed for main channels, which conforms to a bucket width of small dragline excavating equipment. Bottom widths were selected as narrow as design and construction criteria would permit, so as to obtain higher velocities which, in many instances due to low gradients, were not high enough to prevent formation of sediment islands and growth of vegetation in channel bottoms.

Side Slopes

Maintenance methods, soil characteristics, and a need for adequate but economic minimum rights-of-way determined the side slopes of channels. Side slopes of 1 to 1 for main channels were used to satisfy these conditions.

In fine sands having high water tables, sloughing of side slopes may be expected immediately after excavation. Sloughing will continue until the water table becomes established at the lower level. The problem can be controlled somewhat in wide channels by requiring initial construction of a pilot channel to lower the water table followed by final construction when the channel has been stabilized; or by requiring a maintenance operation to restore design cross section soon after the channel has been stabilized.

Design at Culverts

Culverts obstruct the flow of water in ditches and cause a loss in head. This was considered in designing main channels. The hydraulic gradi-



LATERAL CANAL—This canal shows importance of good construction, alignment and spoil management.

ent, in most cases, was set low enough to keep the profile of the water surface at the culvert during design flow well within the channel cross section in all critical areas.

Talbot's formula was used in determining culvert sizes, at the suggestion of the Berkeley County Road Department, since it is their policy and the policy of the South Carolina State Highway Department to use this formula in culvert design. Talbot's formula is as follows:

A = C + M

where: A = Necessary waterway in square feet

M = Area drained in acres

C = Coefficient (.2 used)

Where culvert sizes exceed 60 inches in diameter, it was found more economical to use 15-foot bridges.

Right-of-way Requirement -Berm Width, Spoil Bank

Spoil may be spread on either side

Factors governing width of rights-of-way can best be understood by consulting Figure No. 2. The principal requirements for berm width include a work area for spoil shaping so as to prevent erosion or spoil material sliding into the canal, provide a way for travel by maintenance equipment, and reduce the load near the edge of ditch banks to prevent sloughing. Where unstable soil conditions require it, and the problem of securing wide easements is not a factor, a 15-foot berm width is optimum. Narrower berm widths are feasible where the spoil is to be shaped and a roadway established on top of the spoil.

Dikes, Conduits and Pumps Needs and Location

An integral feature of the water disposal plan is the establishment of dikes across tidal inlets at selected sites to control tidewater intrusion into the major outlets, and provide a basin behind the dike for runoff storage during periods of high tides and high intensity rainfall. Where the capacity of the storage basin is sufficient to store runoff water during a short-duration storm occurring at high tide, the runoff water can be discharged, during the low tide cycle, through conduits equipped with tide gates or through a low gravity flow section through the pump structure. However, where the storage is limited, the storms prolonged, and prevailing winds result in a relatively high tide level, pumps will be required to maintain a safe level of water in the storage basin to prevent damage in highly developed residential areas.

The combination of dikes, tide gates, and pumps will provide protection during times when the drainage canals cannot discharge by gravity. These conditions occur frequently enough to justify the cost. It is during these times that extensive property damage occurs, usually with resultant disruption of public facilities. (See Figures 3 and 4)

Design Criteria

Depth

Bot. width

Available records indicate that average storm tides (excluding hurricane tides) occur at 8.0 feet above mean low water. Design of dikes,

Berm Natural

ground

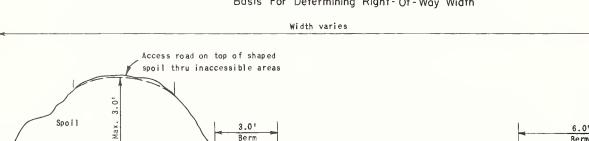


Figure No. 2 - Typical Main Ditch Cross-Section Showing
Basis For Determining Right-Of-Way Width

Figure No. 3 - Typical Profile and Cross-Section - Dike and Pump Structure

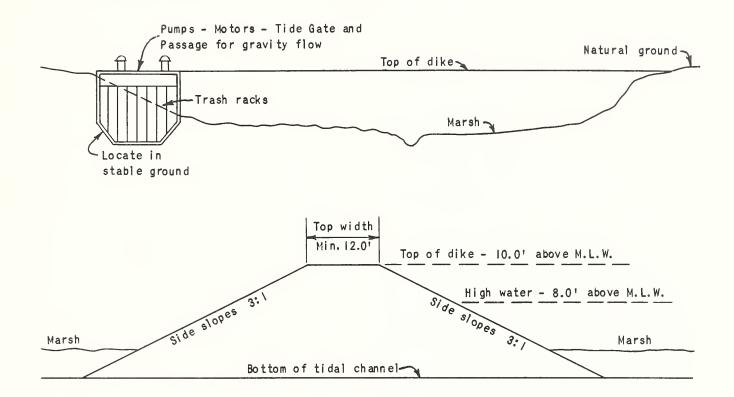
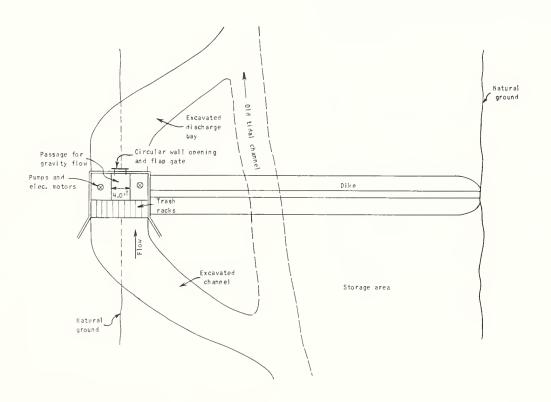


Figure No. 4 - Plan View Showing Typical Installation of Dike - R/C Pump Structure - Tide Gate and Channels



with top elevation of 10.0 feet above mean low water, 3:1 side slopes and 12.0-foot top width, is considered a minimum requirement for adequate protection.

Pumping lift, topography, and foundation conditions are factors which influenced the location of pumps. The axial flow or propeller-type pump was used in determining costs since it is especially adapted for low head pumping.

Three pumps are used for each installation with each pump having one-third the total needed capacity. Adequate trash racks, suction bays, discharge bays, and low-flow gravity chambers are planned.

Reinforced concrete structures for pumps, gates, conduits and trash racks are planned to be located at abutment ends of dikes where good foundation conditions exist. Pumps are planned at an elevation sufficient for protection from inundation during abnormally high tides. Locations are also planned for ease of access and maintenance. (See Figures 3 and 4)



Low head drainage pumps and reinforced concrete pump structure and trash racks.

Description of Areas

These areas are the 13 major watersheds in the county. A brief description of drainage problems associated with each area follows.

Area 1 - Hanahan - Goose Creek - Carnes Crossroads - Oakley

This area, in the southern part of the county, and adjoining North Charleston, is one of the fastest developing areas in the county. It is rapidly changing from agricultural and woodland to residential and industrial use. Bordered on the east by the Cooper River, the land adjacent to the river is being rapidly developed into defense installations by the U. S. Navy. The influx of personnel caused by the U. S. Navy's expanding building programs has created a rapid development of residential and shopping areas. Installation of adequate drainage systems in this area should proceed as rapidly as possible in order to stay ahead of the rapid urbanization

of the area. This will reduce the cost, and eliminate some serious right-of-way problems which might develop. Encroachment of developments on areas exposed to storm tides makes special protective measures such as dikes, tide gates and pumps desirable.

Area 2 - Moncks Corner - Oakley

This area, bounded on the east by the Cooper River and on the north by Lake Moultrie, is one of the most thickly populated areas in the entire county. It includes the town of Moncks Corner, which is the county seat of Berkeley County. The rural part of the area is primarily devoted to agriculture, with a few private tracts of woodland. All drainage in this area flows into the Cooper River since the elevation of Lake Moultrie allows practically no drainage in that direction.

The soils in this area vary from well drained on the higher ridges to very poorly drained along the Cooper River. The subsoils are generally rather tight, allowing slow movement of internal water. In drainage canal design the difference in elevation is such that no difficulty is experienced in securing satisfactory grades in moving water to satisfactory outlets.

Area 3 - Wassamassaw - Cooper's Store-New Hope - Lebanon

Area 3, containing approximately 140 square miles, is the largest watershed area in the county. The entire watershed drains into Cypress Swamp which is the headwaters of the Ashley River. The greater part of this area is woodland, with ownership vested in a Pulp and Paper Company, and other private owners. There are also extensive farming areas in the New Hope and Lebanon sections along the east and west sides of Cypress Swamp.

The soils, excluding the swamp and depressed or bay areas, are among the best in the county and are fairly easily drained. In the woodland areas, some drainage facilities have been installed by a Pulp and Paper Company. These facilities, which provide for drainage and also access roads, are considered adequate in most instances. This area contains several large depressed or bay areas. Some of these are Mosquito Bay, Caton Bay, and Black Tom's Bay. Improved drainage is needed in this area in order to improve agricultural and timber production.

Area 4 - Sand Ridge - Pringletown

This area is in the extreme western part of Berkeley County and is bounded on the west by Four Hole Swamp. It contains several rather small watersheds which drain into Four Hole Swamp. The topography is rather flat with sharp breaks occurring along the edge of the swamp basin. Drainage is, generally, not difficult to obtain as there is usually enough fall in the natural main outlets to obtain satisfactory grades.

Also, this area contains some of the highest land in the county, at one point reaching to an

elevation of approximately 100 feet above mean sea level.

The soils here range from loamy sands on the ridges to sandy loams in the low areas. Internal drainage is satisfactory when adequate outlets are provided.

The area is approximately 60% woodland and 40% cultivated land. Pulp and paper companies own about 60% of the woodland with individual land owners controlling the rest. The agricultural land is devoted to production of several crops such as cotton, corn, tobacco, grain, and pasture.

Area 5 - Cross

Area 5 is largely agricultural. With the exception of woodland areas in the southern part, it is devoted almost entirely to the production of farm crops. Some truck crops are grown, such as beans, potatoes, and cucumbers. Bounded on the north by Lake Marion, and on the east by Lake Moultrie, adequate drainage is difficult to obtain, due to the elevation of the waters in these lakes. The topography is flat and contains many small, shallow bay areas from which surface water cannot escape.

The surface soils range from loamy sands to sandy loams with sandy clay sub-soils. The high water table establishes the need for extensive drainage if cultivation requirements are met and maximum crop yields produced.

Area 6 - Eadytown - Pineville - St. Stephen

This area contains the highest concentration of farms in the county. It is almost entirely agricultural except for the areas along the edge of the Santee River Swamp. Crops grown include: cotton, corn, soy beans, tobacco, and small grains. Some truck crops, including potatoes, beans, and cucumbers, also are grown.

The area is bounded on the north by the Santee River and on the south by Lake Moultrie. A dike along the north side of Lake Moultrie does not allow any drainage into the lake. Therefore, all drainage in this area has to flow north into the Santee River Swamp. Due to the favorable contour of the land, good main outlet canals can be provided in most cases. An exception to this is a small area south of Eadytown and west of the old Santee Canal. Here it will be necessary to dig through a fairly high ridge in order to drain into the Santee River basin.

Area 7 - St. Stephen - Alvin - Jamestown

Area 7 is bounded on the north by the Santee River. All lands in this area drain into this river or its tributaries. The individual watersheds are comparatively small and the required main canals and laterals are, also, comparatively small. The somewhat rolling land and the difference in elevation between the upper and lower ends of the natural draws allow sufficient grades for the proposed canals to remove excess water satisfactorily.

This area is approximately 85% woodland and 15% farm land. National Forest Lands make up approximately 50% of the woodlands, with Pulp

and Paper Companies and individuals owning the remaining 50%.

The soils in this area are rather tight, and internal drainage is slow, especially in the low areas. Adequate drainage is necessary for maximum crop production.

Area 8 - Russellville - Bonneau - Macedonia

This area is located in the north central part of Berkeley County. The northern boundary is roughly a line running east from Russellville through St. Stephen to a point 5 miles east of St. Stephen. The western boundary is a dike along the east side of Lake Moultrie. All drainage in this area flows south into Wadboo Swamp and eventually into the West Branch of the Cooper River. The topography is rather flat but there is sufficient fall in the natural drainage pattern to obtain satisfactory drainage.

The soils range from well drained on the ridges to very poorly drained in the lower areas. Most of these soils have good internal drainage when adequate outlets have been provided.

Cultivated land makes up approximately 20% of the area with the remaining 80% in woodland. National Forest lands make up approximately 50% of the woodland areas.

Area 9 - Macedonia - Bethera - Gough

Approximately 75% of this area is National Forest land and is owned by the Federal Government. About 20% is also in woodland owned either by Pulp and Paper Companies or other private landowners. The remaining 5% is cultivated or pasture lands.

All drainage in this area runs into Wadboo Swamp which is the headwaters of the western branch of the Cooper River. Adequate drainage canals will not be difficult to construct due to there being sufficient fall in the existing natural drains.

The soils are moderately to poorly drained with the better soils occurring along the higher ridges.

Area 10 - Childsbury - Cordesville-Witherbee

For the most part, this area is totally devoted to timber productions. The only exceptions to this are several plantations along the Cooper River and small farms along the main roads such as State Routes 402, 171, and 44.

Most of the land in this area is privately owned either by individuals or by Pulp and Paper Companies. An exception to this is a portion in the northern section which is within the National Forest boundary.

These lands and tidal rivers and creeks furnish an abundance of recreational activities. The woodland areas contain excellent deer, quail, and turkey hunting, while the river and creeks furnish some of the best fishing available. Considerable income is derived by the property owners from organized clubs who lease the property for hunting or fishing privileges.

The contour of the land is rather flat and

interspersed with numerous small bay and pond areas. However, the natural topography is favorable for the installation of needed drainage.

Area 11 - Jamestown - Shulerville - Honey Hill

This area is in the extreme eastern part of Berkeley County. It is bounded on the north by the Santee River and on the south and east by Wambaw Creek. These natural outlets receive all of the drainage from the entire area.

Timber growing is the source of most of the income produced here, about 95% of the total area being in woodlands. Farming is limited to small tracts of land located along State Route 45 from Jamestown to Honey Hill, and in the Shulerville section.

The soils vary from deep, wet sands along the higher ridges to loams with clay subsoils in the depressed bays. Good drainage is practically non-existent even in the farming sections.

A wildlife production and management project is located on a 16,000-acre tract of National Forest land along Wambaw Creek and the Santee River. The purpose of this project is to improve wild game hunting. It is jointly financed by federal and state funds and is administered by the South Carolina Wildlife Resources Commission and the U. S. Forest Service.

Area 12 • Bethera • Huger • Eccles Church-Green Bay

Area 12 presents one of the most difficult drainage problems in the county. The topography is generally flat with large, dense bay areas which have never been drained. Except for the slightly higher ridges surrounding these bays, the area is poorly suited to trees.

National Forest lands comprise approximately 85% of the area, with Pulp and Paper Companies and individual property holders owning the remaining 15%. Practically no farming operations are carried on.

A complete system of adequate main outlets and lateral canals needs to be constructed before the full production of the land capability can be reached. The soils are well suited to trees when proper drainage is furnished.

The three main drainage areas are Nicholson Creek, Turkey Creek, and Quenby Creek, all of which flow into the eastern branch of the Cooper River. Elevations are such that adequate drainage can be obtained.

Area 13 - Pompion Chapel - Charity Church - Wando - Daniel's Island

This area is in the southern part of Berkeley County. It is bounded on the west by the Cooper River, on the north by the east branch of the Cooper River, and on the south and east by the Wando River. The land is influenced greatly by the many tidal creeks which flow into both rivers. The southern part (Daniel's Island) is extremely low and flat, the average elevation being only 8 to 10 feet above mean low water.

The soils are generally poorly drained except those located on the ridges. Outlets have never been provided in most cases.

The area is almost entirely devoted to woodland. There is a small farming section near Charity Church and a few other scattered farms which are engaged principally in cattle growing.

Large, individually owned, woodland holdings make up about 85% of the area, with National Forest lands comprising the remaining 15%.



Right-of-way clearing for Main Drainage Canal.

Soil Associations Description of Soil Associations and Their Drainage Problems

After studying the soils in a locality and the way they are arranged, it is possible to make a general map that shows the main patterns of soils, called soil associations. Such a map is the general soil map in the back of this report. Each association, as a rule, contains a few major soils and several minor soils, in a pattern that is characteristic, although not strictly uniform.

The soils within any one association are likely to differ greatly among themselves in some properties; for example, natural drainage, texture, permeability, or slope. Thus, the general map does not show the kind of soil at any particular place, but a pattern that has in it several kinds of different soils.

The soil associations are named for the major soil series in them, but, as already noted, soils of other series may also be present. The major soil series of one soil association may also be present in other associations but in a different pattern.

The general soil map showing patterns of soils is useful to people who want a general idea of the soils, who want to compare different parts of a county, or who want to know the possible location of good-sized areas suitable for a certain kind of farming or other land use.

Soil Association 1 (Map Symbol 1)

<u>Chewacla-Wehadkee association</u>: Nearly level somewhat poorly drained to poorly drained soils on flood plains.

This association is a nearly level floodplain along the Santee River and occupies 4 percent of the county. The Chewacla soils are 10 to 20 inches higher in elevation than the Wehadkee soils. Chewacla soils comprise 60 percent of this association. They have brown silty clay loam surface layers and yellowish-brown to light yellowish-brown silty clay loam to silty clay subsoils. Wehadkee soils are in sloughs and drainageways and have dark gray silty clay loam surface layers and gray silty clay to clay subsoils. They comprise 40 percent of the association.

This association is well suited to bottomland hardwoods and cypress, having been improved since 1941 by flood reduction and water diversion from Santee River into Lakes Marion and Moultrie.

This association, because of flooding hazard and soil conditions, is unsuited or very severely limited for residences, industry and transportation uses. Agricultural uses are likewise limited. Switch cane grows in abundance and is grazed by cattle and deer. Areas, if adequately drained and protected from flooding, can be expected to respond well to good management.

This association, although not well suited for developed recreation, is well suited in its natural condition for ducks, deer and wild turkey. The streams and lakes in the area afford excellent fishing, and nationally known camps are in the vicinity of these.

Soil Association 2 (Map Symbol 2)

 $\underline{\underline{Swamp}\ association}\colon$ Nearly level, very poorly drained, flooded bottomlands.

This association consists of long, narrow, level, flooded areas bordering Cypress Creek and in Four Hole Swamp. It is about 3 percent of the county area.

The surface layer of swamp soils ranges from sand to clay loam and is dark gray to black. Loam and organic surface layers are in some of the backwater and depressed areas. Subsoils are gray and range from sand to clay. Sandy clay is prevalent.

This association is suited to swamp hardwoods and cypress. Use for this kind of forest is expected to continue. Flooding very severely limits or prevents this association from being used for agriculture, residences, industry, road sites and developed recreation. It is well suited for the production of duck, deer, and wild turkey, especially where provision is made for food. Fishing is good in the streams. This association is a part of many organized hunting club areas leased by landowners as a source of income

Soil Association 3 (Map Symbol 3)

<u>Coxville-Portsmouth association</u>: Nearly level, poorly to very poorly drained soils with clay to sandy clay loam subsoils.

This association is a broad, nearly level area with numerous oval-shaped depressed bays. The bays are 1 to 2 feet lower than the plain and have a northwest-southeast orientation. The association is about 11 percent of the county and includes Pigeon Bay, Ferguson Bay, Mosquito Bay and Catons Bay.

Coxville soils comprise about 50 percent of the association and are in the broad level flats. They are poorly drained and have a dark gray surface layer and a gray sandy clay subsoil. Portsmouth soils are 40 percent of the association and are in the drainways and the more depressed parts of the bays. They are very poorly drained and have a black loam surface layer and a gray sandy clay subsoil. Minor soils comprising 10 percent of the association include Goldsboro, Lynchburg and Dunbar. Hellhole Bay and Pigeon Bay have peat and muck soils.

About 90 percent of the area is woodland and the remainder is cropland and some pasture. It varies from excellent to poor for pines and hardwoods because of high water tables and standing water. Corn, tobacco, and cotton are grown on the better drained minor soils.

Intensive drainage is required if the Coxville and Portsmouth soils are to be used for farm crops.

The association has severe limitations for residences using septic tanks, commercial and transportation use. Some agricultural uses are limited, due to inadequate outlets for drainage. Foods for quail and deer are plentiful. Landowners either hunt and/or lease hunting rights as a source of income.

Soil Association 4 (Map Symbol 4)

Goldsboro-Norfolk-Portsmouth association: Nearly level to depressed, moderately well drained, well drained, and very poorly drained soils. This association is a nearly level plain with numerous small, oval and elongated, narrow wet depressions or bays oriented in a general north-south direction.

This association is in the northwest part of the county and is 3 percent of the county.

Goldsboro soils are 60 percent of the association and are on the broader, nearly level areas. They are moderately well drained and have a dark grayish-brown loamy sand surface layer and a slightly mottled sandy loam to sandy clay loam subsoil. The Norfolk soils comprise 15 percent of the association and are on higher narrow areas. They are well drained and have a graybrown loamy sand surface layer and a yellowishbrown sandy loam to sandy clay loam subsoil. The Portsmouth soils are 15 percent of the association and are in the depressions. They are very poorly drained and have black loam surface layers and gray clay loam subsoils. Lynchburg, a somewhat poorly drained soil, and Lakeland, a dry sandy soil, are minor soils making up the remainder of the association.

About 65 percent of the association is in cultivation and 30 percent in pine and pine-hardwood forests. The remainder is improved pasture. It varies from poor to excellent for the production of pine and commercial hardwoods.

Most of the grazing is in idle fields, and cultivated fields after harvest, and in woodlands. The average farm is about 35 acres in size and is owner-operated. The main income is from cotton, corn, and some tobacco.

The association is suitable for farming. It has moderate limitation for residences requiring septic tanks, road sites, and industrial sites. The wetter areas have severe limitations for these uses. Fishing and quail hunting under good management can be maintained at a high level. The area has much good cover and there are many natural and cultivated plants that are productive as quail foods. Hunting-right leases are good sources of income, as are cabins and cabin sites for rent for fishermen.

Soil Association 5 (Map Symbol 5)

<u>Eulonia-Weston-Edisto association</u>: Nearly level, moderately well drained to somewhat poorly drained soils with sandy clay loam to clay subsoils.

This association is a broad, nearly level area with a random drainage pattern leading to tidal marshes. Most of it is less than 15 feet above sea level and ranges from 5 to 25 feet.

The association is about 2 percent of the county and is in the Goose Creek Reservoir and Daniels Island part of the county.

Eulonia soils are about 35 percent of the association and are on the highest areas. They are moderately well drained and have a dark grayish-brown loamy sand surface soil and a yellowish-brown sandy clay subsoil. Weston soils are about 30 percent and are in low positions along the intermittent drainageways. Edisto soils are about 20 percent of the association and are intermediate in elevation between the Eulonia and Weston soils. They are somewhat poorly drained and have a dark grayish-brown sandy loam surface layer and a yellowish-brown sandy loam subsoil. The Bladen, Fairhope and Wando soils are minor and are about 15 percent of the association. Bladen is poorly drained and is in the lowest areas bordering streams and tidal marshes. Fairhope soils are well drained, gently sloping soils. They have red compact sandy clay subsoils. Wando is a deep droughty sand.

About 40 percent of the association is woodland, 20 percent pasture, 10 percent cropland and 30 percent in urban and industrial use. The areas is excellent for production of pine and hardwoods. Areas that can be drained adequately are being developed for housing and commercial sites. Streams in the area are excellent for fishing and water skiing. Sections of this association are used for hunting deer and quail. Wildlife food is plentiful.

Soil Association 6 (Map Symbol 6)

Bladen-Weston-Eulonia association: Level to nearly level, poorly drained to moderately well drained loam to loamy sand soils with clay to sandy clay loam subsoils.

This association is a long, narrow, nearly level plain crossed by numerous shallow drainageways. Flooding is frequent for about half its

length where it borders Wadboo Creek and West Branch Cooper River. The southern half is not flooded because it is higher and farther from the river. The association is about 2 percent of the county.

Bladen soils occupy 60 percent of the association and are on a level floodplain bordering the creek and river. They are poorly drained and have very dark gray sandy loam to loam surface layers and mottled gray fine sandy clay or clay subsoils. The Weston soils are 25 percent of the association and are on the nearly level, slightly higher parts seldom flooded by the river. They are somewhat poorly to poorly drained soils with very dark gray loamy sand surface layers and mottled sandy clay loam to sandy clay subsoils. Eulonia soils are 10 percent of the association and are on the highest areas, well above flood stage. The remainder of the association includes the Edisto and Fairhope soils on the higher areas and Bayboro on the floodplain.

Over 90 percent of the association is suited to hardwoods, including scattered areas of pine-hardwoods on the higher unflooded part. The remaining 10 percent is mostly residential and built-up areas. The association will probably remain in woodland, with an increase in residential and industrial development on the higher areas.

That part of the association subject to flooding (about 50 percent) is unsuited for uses other than forest, and much of the remainder has severe limitations for agriculture, residences requiring septic tanks and industrial uses. Severe limitations exist on the areas above flooding levels. Many plantations border this association and recreational facilities are being developed on them. The area furnishes good quail and deer hunting. Woodland duck ponds can be made by diking along small streams. Nearby streams furnish excellent fishing.

Soil Association 7 (Map Symbol 7)

Norfolk-Lynchburg-Coxville association: Well drained to poorly drained loamy sand soils with sandy clay loam to clay subsoils on narrow, nearly level ridges and flats.

This association consists of narrow nearly level parallel ridges and narrow to broad level flats and narrow drainageways. The ridges are 500 to 1,000 feet wide and 3 to 6 feet above the flats and drainageways. There are a few short steep slopes along the larger creeks. The flats are 1,000 to 2,500 feet wide. The association is 24 percent of the county area.

The Norfolk soils are 35 percent of the association and are on the ridges. They are well drained and have a grayish-brown loamy sand surface layer and a yellowish-brown sandy clay loam subsoil. The Lynchburg soils are 30 percent of the association and are on the lower side of the ridges and nearly level parts of the flats. They are somewhat poorly drained and have a dark gray to black loamy sand surface layer and a mottled light brownish-gray to light gray sandy loam subsoil. Coxville soils are 25 percent of the association and are on the flats and bays. They are poorly drained and

have a very dark gray to black sandy loam surface layer and a mottled gray sandy clay subsoil. The remainder of the association consists of Goldsboro, Klej and Lakeland on the intermediate and higher parts and Portsmouth in the drainageways.

About 30 percent is in cultivation, 5 percent in pasture, and the remainder in pine and pine-hardwood forests to which it is moderately to well suited. The trend is towards more pasture and woodland. The average farm is about 60 acres in size. Most farms are owner-operated. Main income is from cotton, tobacco, and soybeans, with secondary income from hogs.

In the western part of the county, cotton, corn, and soybeans are the important crops. In the northern part, cotton, tobacco, soybeans, and corn are the chief crops. In that part east and south of Lake Moultrie, corn is the main crop.

This association has slight limitations on the better drained soils to severe on the wetter soils, for agriculture, woodland, residence, industrial and transportation uses. Recreation benefits are attractive for fish and wildlife revenue. Lake Moultrie and the many natural ponds afford good fishing and there are many natural draws that can be diked for fish ponds. There are good cover and food resources for quail, deer and turkey. Duck hunting is good.

Soil Association 8 (Map Symbol 8)

<u>Tidal Marsh</u>: Wet peats, mucks, and loams flooded by tide water.

This is a treeless plain intricately dissected by meandering drainageways and flooded daily by tidewater. Most of the association is at about mean sea level but ranges from 5 feet above to 5 feet below sea level. It is about 3 percent of the county and borders the Wando River and lower reaches of the Cooper River.

Most of the tidal marsh is soft clay having a black oozy clay surface layer underlain by gray soft clays. It has a thick, tall growth of salt-tolerant grasses. This is called Tidal marsh, soft. Small areas of firmer marsh (Tidal marsh, firm) occur where tide water does not cover the marsh as deep or as long as on soft marsh. It has a dark gray surface layer and a gray clay, sand, or sand-clay subsoil. The marsh vegetation is 6 to 24 inches tall and less thick than the soft-marsh grasses.

The salt content of the water prevents the growth of vegetation except the salt-tolerant grasses. If drained, the soil develops into an extremely acid plastic clay commonly called "cat clay". All vegetation is killed and the soil is difficult to reclaim.

Flooding, poor internal drainage, potential for becoming extremely acid, and low bearing capacity are limitations that give tidal marshes very severe limitations for residences, industrial and transportational use, and agriculture. These marshes furnish some facilities for fishing and crabbing. They may be managed as duck ponds if water control systems, including dikes, are constructed. Widgeon grass, as food for ducks, grows well where the water is brackish.

Soil Association 9 (Map Symbol 9)

Fresh Water Marsh: Very poorly drained, clayey soils flooded by fresh water.

This is a level, treeless flooded plain bordering the east and west branches of the Cooper River. Elevations range from 5 to 8 feet above sea level. The association is about 2 percent of the county.

The soils are unclassified. The surface layer is dark gray to black soft clay underlain by gray firm clay. Reeds, rushes, cattails, and fresh water marsh grasses grow in the shallow water areas.

Flooding prevents its use for residential, industrial, transportational, and agricultural use. At one time this association was used extensively for rice. The old rice lands are well suited for development of fish and duck ponds.

Soil Association 10 (Map Symbol 10)

<u>Plummer-Rutlege association</u>: Poorly drained and very poorly drained wet sands in depressions and low, nearly level flats.

This association is a broad wooded plain with numerous small to large oval shallow depressions (Carolina Bays) with a northwest to southeast axis. The larger ovals have a low narrow sand rim on the southeast side. The small ovals range from a few hundred feet to 2,500 feet in length. Large ovals are a mile to 2 miles long. The small ones are more numerous. This association occupies about 1 percent of the county.

Plummer soils are 50 percent of the association and are on the slightly higher, nearly level areas along the inside edge of the bays. They have thin black sand surface layers and loose light gray sand subsoils. Rutlege soils are 40 percent of the association and are in the permanently wet, depressed, central parts of the bays and along the drainageways. They have black loamy sand to loam surface layers about 8 to 12 inches thick underlain by light gray loose sand. The remainder of the association consists of Klej and Lakeland soils on the rims and shallow organic soils in the deeper depressions.

The association, because of extreme wetness, is suited for pond pine, cypress, and bottomland hardwoods. Areas with water management are producing loblolly and slash pine. The association is unsuited for residence, industry, or agricultural use. Drainage improves the suitability for crops and pasture.

This association is quite productive of wildlife. The wet areas produce some food for wildlife and give excellent cover. Practically all of the acreage is leased for hunting rights for deer, duck, and other wildlife.

Soil Association II (Map Symbol 11)

Craven-Duplin-Dunbar-Coxville association: Moderately well drained to poorly drained slightly undulating to level soils with sandy clay loam to sandy clay subsoils.

This association is dominantly upland flats with ill-defined sand on drainageways. It

occupies 40 percent of the county and comprises much of the eastern half of the county.

Craven soils are 20 percent of the association and are on the highest areas. They are moderately well drained and have dark gray fine sandy loam surface layers and yellowish-brown clay subsoils. Duplin soils are 18 percent of the association and are on the highest areas. They are moderately well drained and have dark gray fine sandy loam surface layers and yellowishbrown to pale brown clay subsoils. Dunbar soilsare 35 percent of the association and are on the nearly level parts adjacent to the Duplin soils. They are somewhat poorly drained and have very dark gray fine sandy loam surface layers and clay subsoils. Coxville soils are 20 percent of the association and are in the drainageways and flats. They are poorly drained and have very dark gray to black fine sandy loam surface layers and gray clay subsoils. The remainder of the association consists of Caroline soils on the slopes. Bayboro and Portsmouth in the depressions and Goldsboro on somewhat sandy ridges. Deep sand soils, Lakeland and Klej occur in the Honey Hill section.

About 15 percent of the association is cropland and pasture - the remainder is woodland. The area is well to poorly suited to pine and/or hardwoods. About half of the association is in the Francis Marion National Forest. The use trend is toward more pasture and woodland. The average agricultural farm is 75 acres in size and is owner-operated. Plantations managed as tree farms border the Cooper River and its branches and range from 1,000 to 3,000 acres. A few woodland company tracts exceed 3,000 acres.

The main farm income is from cotton, some tobacco, soybeans, corn and livestock.

The association is fair for farming and the soils respond well to drainage. They have moderate to severe limitations for industrial use and for residences that require septic tanks.

Under good management, this association is productive of good plants for quail, deer, and turkey. Field crops, bicolor lespedeza, and other plants are among the more productive food plants. Landowners either hunt and/or lease hunting rights as a source of income.

Soil Association 12 (Map Symbol 12)

<u>Leaf-Wahee-Flint association</u>: Poorly drained to moderately well drained soils on nearly level to gently undulating stream terraces.

This association is a nearly level, long, narrow band along the west side of the Santee River and is flooded less frequently than river bottoms. The association is 4 percent of the county area.

The Leaf soils, making up 45 perent of the association, are on the level to depressed areas and along drainageways. They are poorly drained and have black fine sandy loam surface layers and gray mottled clay subsoils. Wahee soils are 35 perent of this association and are on the nearly level areas slightly higher in elevation than Leaf soils. They have dark gray to gray fine sandy loam surface layers and mottled light gray, yellow and reddish-yellow silty clay subsoils. Flint soils are 20 percent of the asso-

ciation and are the nearly level highest areas, knolls and gentle slopes. They are moderately well drained and have gray to dark brown shallow fine sandy loam surface layers and yellowish-red clay subsoils.

About 3 percent of the association is in crops and pasture. The remainder is in woodland. About 85 percent is privately owned and the remainder of the association is in Francis Marion National Forest. General farm crops and pasture grasses are grown on the open land. The association is only fair for farming because of wetness. It has moderate to severe limitations for building sites and industrial purposes. It has severe limitations for residences requiring septic disposal systems.

This association is well suited for pine and hardwoods and recreational uses. The banks next to the river on the higher elevations make ideal cabin sites. The adjoining river offers good fishing. The draws are good sites for duck and fish ponds. Deer and turkey foods are plentiful. Hunting is excellent.

Factors Considered in Preparation of Plan

The Drainage Feasibility Study was prepared by engineers of the Soil Conservation Service with the assistance of the Berkeley County Supervisor's Office and U. S. Forest Service. On-site investigations were made of the outlets for each main canal, and the factors affecting drainage within the watershed, such as tidal ranges, river stages, flooding, and the time of year in which flooding occurs, were studied.

The soil association description of the county, prepared by Soil Scientists of the Soil Conservation Service, was used to determine the soil characteristics which affect drainage design and construction. Present land use and anticipated future land use was considered in preparing the design of drainage canals. Engineering information available through the Berkeley Work Unit Office of the Soil Conservation Service was also used, particularly that pertaining to drainage investigations.

U. S. Geological Survey Topographic Maps were used to determine the general topography within each watershed and to assist in delineation of watersheds. A limited amount of instrument surveying was made to secure detailed information in critical areas.

Aerial photographs, scale 1'' = 1320', flown in 1957-58, were used in recording field data and for the preparation of the drainage plan.

Agencies and commercial concerns, having knowledge of specific drainage problems, were consulted in making the final decisions in certain areas. Also, maps, surveys, and plans available from these agencies were used.

In most instances, mains were located along natural drainageways with modifications in alignment to improve the flow and the collection of water. All needed laterals within the watersheds were not located since the purpose of the study is to locate and design only the main canals which will furnish the means for disposal of runoff from all parts of the watershed. All

mains are terminated in tidal creeks or natural outlets at a point where they have adequate capacity and depth.

No attempt was made to locate underground utilities such as cables, gas pipe lines, water mains, and conduits. However, due consideration must be given to the location of these underground utilities during the preparation of the final plans.

In general, the drainage plan was limited to areas considered as "high lands", that is, five feet or more above mean low water.

Drainage plans were not prepared for areas in the southern part of the county owned by the U. S. Navy. These lands, including the Naval Ammunition Depot and the Polaris Missile Base, were omitted due to the fact that it is their policy to handle their own drainage problems.

Watersheds draining into the county from adjoining counties were determined for the purpose of designing main canals. The mains, however, are shown beginning at the county line. Due attention was given to possible land use changes which would affect runoff within the portion of these watersheds in adjacent counties.

Engineering Considerations

Engineering considerations for planning, design, construction, maintenance and other matters pertinent to the Main Drainage Canals Feasibility Study are listed below:

Design

- The plan presented herewith is a <u>Feasibility</u> <u>Study</u> to estimate the cost and the extent of needed main drainage facilities and the physical practicability of drainage in the county. Detailed engineering surveys and designs will be required before any part of the proposed plan is constructed. All improvements should be made continuous, beginning at the lower or outlet end of the watershed.
- 2. Plans and designs contained in this report do not include a complete study of underground storm sewers found in Areas 1, 2, and 6, due to the fact that these are not considered as mains. Also, there is a lack of information on original surveys and designs showing size, depth, and location. Detailed studies will be needed to determine the present condition of these storm sewers and their additional needs.
- 3. Culverts at rail and road crossings were designed to satisfy the minimum requirements based on expected flow. Increases in size of these structures may be desirable to provide an added safety factor for passing runoff in excess of designed flow; especially, where presently unforeseen improvements are made in the vicinity.
- 4. The South Carolina Wildlife Resources Department should be consulted when fish and wildlife may be affected by the construction of main drainage canals.

Acquisitions of Rights-of-way

The means for, and the acquisition of, adequate rights-of-way for the installation of main canals is absolutely essential. The width of the right-of-way must be adequate to take care of spoil management, channel cross sections, berm, and access. (See Figure 2)

Maintenance of Channels

A well organized and adequately financed maintenance program is essential to maintain design capacity in all canals. Provision for annual maintenance or periodic reconstruction to maintain the effectiveness of the channel must be considered prior to construction. The failure of many drainage enterprises to function as designed can be directly attributed to an inadequate maintenance program. Maintenance of the designed depth of channels is one of the most important items in a maintenance program. The cost of maintenance may be reduced considerably if provision is made in channel designs for easy access, stabilization of side slopes and other silt-contributing areas such as road fills and road drainage immediately following construction. Provision should also be made for maintenance of pumps, conduits, tide gates and dikes, so that these installations may be completely operable at all times.

Obstructions

Construction of fences, walks, and other structures that may retard channel flow should not be permitted except as approved by the responsible agency of the County Government. Other structures such as culverts, bridge piers, trestles, etc., should be designed so as to cause minimum interference with the channel flow. Dumping trash, garbage, and other debris in channels should be prohibited.

Definition of Terms

c.f.s. - Abbreviation for cubic feet per second; a unit of water-flow - sometimes call "second feet".

<u>Infiltration</u> - The entrance of water into surface horizons or sola.

Internal Drainage - The movement of water through the soil profile. The rate is affected by the texture of the surface soil and of the subsoil and by the height of the water table. A wet, deep sand may have slow internal drainage when the water table is high, and rapid internal drainage when the water table is low. A plastic, sandy clay soil may have slow internal drainage regardless of water table height.

<u>Lateral Ditch</u> - A major ditch in a drainage system which serves as a link between the main ditch and the collection system in a segment of the watershed. Main Canal (Ditch or Channel) - The principal channel which conducts the drainage water from the watershed to the outlet.

<u>Permeability Rate</u> - The rate of movement of water through the soil

<u>Profile, Soil</u> - A vertical section of the soil through all its horizons and extending into the parent material.

Reach - A length of channel selected for use in hydraulic computations.

Relief - The elevations or inequalities of a land surface, considered collectively.

<u>Runoff, Surface</u> - Total rainfall minus interception, infiltration, and surface storage, that which moves across the ground to a stream or depression.

<u>Runoff, Subsurface</u> - Water that infiltrates the soil and reappears as seepage or spring flow.

Soil Drainage - (1) The rapidity and extent of the removal of water from the soil by runoff and flow through the soil to underground spaces. (2) As a condition of the soil, the frequency and duration of periods when the soil is free of saturation. For example, in well-drained soils, the water is removed readily, but not rapidly; in poorly drained, the root zone is waterlogged for long periods and the roots of ordinary crop plants cannot get enough oxygen; and in excessively drained soils, the water is removed so completely that most crop plants are damaged by lack of water.

<u>Soil Structure</u> - The arrangement of the individual grains and aggregates that make up the soil mass; may refer to the natural arrangement of the soil when in place and undisturbed or to the soil at any degree of distrubance.

<u>Subsoil</u> - In soils with weak profile development, the subsoil can be defined as the soil below the plowed soil (or its equivalent of surface soil) in which roots normally grow.

<u>Surface Soil</u> - The soil ordinarily moved in tillage or the equivalent in uncultivated soil about 6 to 10 inches in thickness.

Terrace (Geological) - An old alluvial plain, ordinarily flat or undulating, bordering a river, lake, or the sea. Stream terraces are frequently called second bottoms, as contrasted to flood plains, and are seldom subject to overflow. Marine terraces were deposited by the sea and are generally wide.

Texture, Soil - The relative proportions of sand, silt, and clay particles in a mass of soil. The basic textural classes, in order of increasing proportions of fine particles are as follows: sand, loamy sand, sandy loam, loam, silt loam, silt, sandy clay loam, clay loam, silty clay loam, sandy clay, silty clay, and clay. The sand, loamy sand, and sandy loam classes may be further divided by specifying "coarse", "fine", or "very fine". A coarse textured soil is one high in sand content; a fine textured soil is one high in clay content.

Tide Data

Mean Range - Difference between mean high water and mean low water.

Spring Range - The average range which occurs semi-monthly as a result of the moon's being full or new.

Mean Tide Level - (Half tide level) - is a plane midway between mean low water and mean high water.

High Water - The maximum height reached by each rising tide.

<u>Water-holding Capacity</u> - The ability of a soil to hold water. The capacity (or ability) of soil to hold water against gravity.

<u>Watershed</u> - An area of land from which all water that falls within the area, converges toward and discharges past a designated point.

TABLE NO. 2
SUMMARY OF DATA AND ESTIMATED COST
DIKES - TIDE GATES AND PUMPS

Area No.	Pump Site No.	Main Canal No.	Dike Length - Ft.	No.	Pumps Capacity - Ea GPM	Estimated Total Cost
1	А	M-18	700	3	50,000	\$ 65,000.00
GRAND	TOTAL					\$ 65,000.00

TABLE NO. 3
SUMMARY OF ENGINEERING AND DESIGN DATA BY AREAS

1

Area	Length Canals and Laterals Feet	Excavation Cubic Yards	Right-of-Way Clearing Acres	Dike and Pump Installations Number	Estimated Total Cost Dollars
1	581, 100	1,301,578	562 .4	1	433, 218.00
2	258, 300	480, 695	222.8	-	179,513.00
3	794, 000	1,962,278	850.9	-	652, 137.00
4	284, 900	464, 017	222.7	-	163,528.00
5	295, 000	478, 126	229.2	-	181,713.00
6	413,000	715, 303	339.6	-	271,697.00
7	432,800	936,793	417.3	-	308, 554.00
8	543, 300	1,424,379	605.7	-	469, 186.00
9	460, 900	1,080,339	471.4	-	337,988.00
10	426,800	657, 997	321.1	-	228, 523.00
11	573, 400	1,655,244	690.2	-	515, 188.00
12	887,800	1,536,681	726.8	-	514, 313.00
13	545, 200	934, 663	435.3		314, 504.00
COUNTY TOTALS	6, 496, 500	13,628,093	6,095.4	1	4,570,062.00

Technical References

- C. F. Ramser FLOW OF WATER IN DRAINAGE CHANNELS U. S. Department of Agriculture -Technical Bulletin No. 129 - U. S. Government Printing Office - Washington, D. C.
- H. W. King HANDBOOK OF HYDRAULICS McGraw-Hill Book Co., Inc., New York, N. Y.

War Department, Corps of Engineers - HYDRAULIC TABLES - U. S. Government Printing Office - Washington, D. C.

- U. S. Department of Agriculture, Soil Conservation Service NATIONAL ENGINEERING HANDBOOK - DRAINAGE - Section 16, Chapters 1, 2, 3, 4, 5, 6.
- U. S. Department of Agriculture, Soil Conservation Service NATIONAL ENGINEERING HANDBOOK - HYDRAULICS - Section 5.
- U. S. Department of Agriculture, Soil Conservation Service FIELD DRAINAGE GUIDE FOR SOUTH CAROLINA.
- U. S. Department of Commerce, Weather Bureau TECHNICAL PAPER NO. 4 RAINFALL, FREQUENCY ATLAS OF THE UNITED STATES - U. S. Government Printing Office Washington, D. C.
- U. S. Department of Agriculture, Soil Conservation Service NATIONAL ENGINEERING HANDBOOK - HYDROLOGY - Section 4.

Feasibility Study for Main Drainage Canal in Charleston County.

Authority and Acknowledgement

Authorization for preparation of the Feasibility Study of Requirements for Main Drainage Canals for Berkeley County is the result of a cooperative agreement entered into on March 4, 1963 by:

Berkeley County - Rembert C. Dennis, State Senator

Henderson Guerry, Member of House of Representatives

H. N. West, Member of House of Representatives

Berkeley Soil Conservation District - N. L. Harvey, Chairman

Soil Conservation Service - T. S. Buie, State Conservationist

U. S. Forest Service - P. H. Russell, National Forest Supervisor

Direct responsibility for preparation of Plans, Designs and Final Report was as follows:

- R. Molinaroli Civil Engineer, Soil Conservation Service
- C. C. Allen Civil Engineer, Soil Conservation Service

For Berkeley County:

Clyde D. Umphlett - Berkeley County Supervisor

Soil Association Map and Descriptions by:

James E. McDonald - Soil Scientist, Soil Conservation Service

F. F. Lesesne - Soil Scientist, Soil Conservation Service

Special technical assistance during all phases of the preparation of the report was given by:

- J. L. Aull State Conservation Engineer, Soil Conservation Service
- E. A. Schlaudt Drainage Engineer, Soil Conservation Service
- C. M. Ellerbe State Soil Scientist, Soil Conservation Service
- J. S. Livingston Work Unit Conservationist, Soil Conservation Service

Others who furnished data, information, or services used in the preparation of this report are as follows:

Berkeley County Road Department

U. S. Forest Service

West Virginia Pulp and Paper Co. U. S. Corps of Army Engineers

U. S. Weather Bureau

South Carolina Public Service Authority South Carolina Highway Department South Carolina Wildlife Resources Department

Fairbanks-Morse and Company

South Carolina Extension Service, Clemson, S.C.

The assistance of J. Clifford Martin, Moncks Corner, S. C., in the completion of this report is gratefully acknowledged.

Cartography and Printing - Spartanburg Cartographic Unit, Soil Conservation Service

Explanation of Engineering Data Tables

The following Engineering Data Tables contain information, by areas, for each main canal and laterals, by watersheds.

An explanation of each column in the Engineering Data sheets is as follows:

- Column 1 CANAL NUMBER

 Numbering of main canals begins

 with M-1 and laterals with L-1,

 in each area.
- Column 2 LENGTH IN FEET

 The stationing of all mains and laterals begins at the upper end (headwaters) and continues toward the outlet. The mains and laterals are shown in reaches or sections in the data tables for design purposes. Each reach, or section, reflects a change in water concentration resulting from the entrance of lateral drainage.
- Column 3 WATERSHED IN ACRES

 See Definition of Terms
- Column 4 DISCHARGE CUBIC FEET PER SECOND

 From appropriate drainage coefficient curves dependent on the land use.
- Column 5 TOP WIDTH IN FEET Self explanatory
- Column 6 BOTTOM WIDTH IN FEET Self explanatory
- Column 7 AVERAGE DEPTH IN FEET Self explanatory
- Column 8 EXCAVATION IN CUBIC YARDS
 Self explanatory
- Column 9 RIGHT-OF-WAY CLEARING IN ACRES Self explanatory
- Column 10 REQUIRED RIGHT-OF-WAY WIDTH IN FEET

 Based on minimum requirements
 for channel cross section, spoil
 management, berm width, and
 access road for maintenance
 equipment.
- Column 11 CULVERTS, LOWERING LENGTH AND SIZE

 Refers to the existing in-place
 culverts which are to be re-used.
- Column 12 CULVERTS AND BRIDGES NEW LENGTH
 AND SIZE

 Refers to additional culverts
 or bridges required to handle
 design discharge. Design of
 culverts is based on round
 concrete pipe.

R.C. Br. - Reinforced Concrete
Bridge
C.T. Br. - Creosoted Timber
Bridge

U.T. Br. - Untreated Timber Bridge

Column 13 TOTAL ESTIMATED COST IN DOLLARS

Total costs shown include only the estimated construction costs and do not include engineering costs and the cost of acquiring required right-of-way. When preparing the final cost estimates these engineering costs and right-of-way costs should be included in the total cost of the project. Total estimated costs as shown are based on the following unit prices prevailing in Berkeley County in 1964.

EXCAVATION

Rural Area - High Ground - - \$0.20 per cu. yd. Urban Areas - - - - - - - - \$0.30 per cu. yd. Marsh - - - - - - - - - \$0.50 per cu. yd.

DIKE EMBANKMENT MATERIAL

In Place - - - - - - - \$1.00 per cu. yd.

RIGHT-OF-WAY CLEARING AND GRUBBING
All Areas - - - - - - \$200.00 per acre

LOWERING EXISTING CULVERTS

Labor and equipment costs only.

NEW CULVERT AND CONDUIT COSTS

Based on present cost of circular concrete pipe.

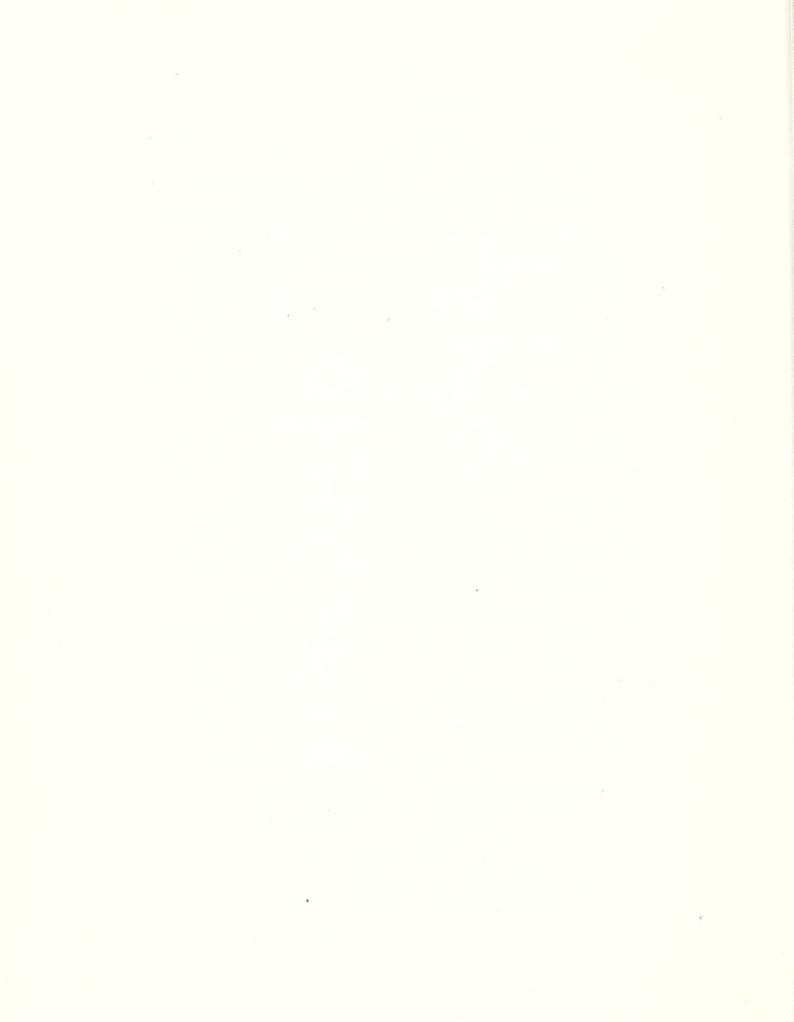
BRIDGES

Three types of bridges were used for design purposes.

- Precast reinforced concrete bridges were used under main highways and secondary roads.
- Pressure-treated creosoted timber bridges were used under National Forest roads and other county roads.
- Untreated timber bridges were used on farm and private roads.

PREVAILING COSTS OF BRIDGES

Reinforced concrete bridges - - \$100.00 per linear foot Creosoted timber bridges - - \$ 50.00 per linear foot Untreated timber bridges - - \$ 33.33 per linear foot



Area 1 - Hanahan - Goose Creek - Carnes Crossroads - Oakley

Sheet 1 of 4	TOTAL	ESTIMATED COST	Dollars (13)								20,306,00														42,300.00		5.591.00			B 041 00			7,097.00									
		CULVERTS & BRIDGES - NEW	Length & Size (12)	ļ	1		1	!	1	1		-	1	1	!	1]	1 1	40' - 24"	I	1	-	1						15' R.C. Br.		15' R.C. Br.			an an	1	15' R.C. Br.		1 5	SO' R.C. Br.	1 !	1	
akley		CULVERTS	Length & Size (11)		,	1 1	1	40' - 24"	1	30.1 - 18"		!	1	1			1 1	20' - 24"	i	1	I	1	1	40' - 18" 40' - 24"		80' - 30"		101		40' - 18"		1		1		-		1				
sroads - O	REQUIRED	RT. OF WAY WIDTH	Ft. (10)	38	200	N C	57	38	38	89 89 87 87)	38	38	38	46	N 0	0 W	38 8	38	38	38	38	38	38		38	38	38	44	38	38	41				55	84	o 0	ω 	137	163	190
rnes Cros		RT. OF WAY	Ac. (9)	2.7	o a	0.	2.6	2.9	4.7	n m	27.2	2.6	0.7	4.0	0.0	φ c	7.4		3.5	2.3	0.9	4.3	υ. Τ	4.0	63.9	4.3	4.8 6.9	0) m	3.1	3.5	3.2	6.7	adequate	adequate	4.0 0.0	10.6	ю. 4. с	n w		14.4	18.2
Creek - Carnes Crossroads - Oakley		EXCAVATION	Cu. Yds. (8)	5476	1828	10,604	6116	7400	9472	6216	58,604	5180	1332	8140	4284	19,039 20 365	18.304	6660	6956	4588	12,136	8732	10,360	10, 932	139,028	8584	8732	8140	7030	6216	7104	6680	13,784		s considered	11,396	27,317	22,000	14,000	43.316	40,760	51,772
- Hanahan - Goose	DINENSIONS	AVERAGE DEP TH	Ft. (7)	ប្រ	טוכ	വ	Ŋ	Ŋ	LO I	വവ)	22	ω	Ŋ	rO H	υи	יו כי	വ	Ŋ	Ŋ	ιΩ	ω	ıo ı	ာ		ιO	Ŋ	ιc	വ	Ю	ιΩ	D		••	ed i	ıo ı	ο ι	υ n	Ωи	טו כ	ro.	го
anahan	EL L	BO T T O W W I D T H	·Ft. (6)	8	ე ლ	ο Φ	10	ო	m	ന ന)	8	ო	ဇာ	ω c	σ c	14	r m	ო	ო	က	ო	ကပ	n		e	ო	ď	വ	ဇ	8	4		s s	ග ස්	o (0 %	222	22	40	20	09
-	CHAN	TOP	Ft. (5)	13	T T	18	20	13	13	133)	13	13	13	16	ρ ς ο ο	24	13	13	13	13	13	13	T		13	13	7.0	15	13	13	14			0	61 0	9 6	n c	32	- L	09	70
Area		DISCHARGE	C. f. 8. (4)	30	7.5	127	152	47	51	34	1	28	45	28	127	n 0 ⊌ 0	310	16	24	13	75	36	48	φ.		61	84	a	50	19	51	63		Present	Present	138	291	310	3 L4	577	692	044
		WATERSHED	Ac. (3)	392	1194	2196	2784	496	740	452	1	356	632	898	2234	5550 5048	6406	188	300	152	1192	484	720	000		918	1360	360	822	236	740	096		1740	2332	2460	0324	6408	0560	13,288	16,410	19,130
		LENGTH	Ft. (2)	3700	1100	4400	2200	2000	6400	4200	34,900	3500	006	5500	2100	7.100	2000	4500	4700	3100	8200	2900	7000	0047	73,000	5800	5900	5500	3800	4200	4800	4000	8800			4400	0060	4400	2800	5200	4000	4300
		CANAL	No.	M-1	M → T	M T-M	M-1	L-1	L-2	L-3	Total-1	M-2	M-2	M-2	W = 2	Z Z	2 C I I W	L-1	L-2	L-3	L-3	L-4	1 L-5	. 0	Total-2	M-3	M-3 Total-3		M - 4	L-1 Total-4	M-5	M-5	Total-5	M-6	M-6	W X	0 (W)	W-0	0 0		M-6	M-6

Sheet 2 of 4 167,889.00 24,223.00 ESTIMATED Dollars (13) COST Br. R. C. Brs. R.C. Br. BRIDGES - NEW R.C.Brs. 2-15' R.C.Brs. Length & Size 15' R.C. Br. 15' R.C. Br. CULVERTS & - 24" 40' - 24" - 48" R.C. (12) 151 15' 30, 404 2-15 2-15' CULVERTS LOWERING Length & Size - 18" - 18" - 24" - 36" 1 1 1 1 1 | | | | Area 1 - Hanahan - Goose Creek - Carnes Crossroads - Oakley 30. 40 4 601 60 102 RT. OF WAY REQUIRED Ft. 38 41 62 62 73 73 41 46 57 62 68 38 38 88 RT. OF WAY CLEARING adequate adequat 10.1 3. 5. 8 3. 51 13 (6) 3.9 30.5 0.0 4.0 220.6 EXCAVATION consider 12,852 Cu. Yds. 11,988 12,024 13,056 8140 13,737 21,105 5365 12,232 2205 28,896 4588 9853 5355 10,656 3885 6808 8288 8016 2960 6660 8288 6956 16,095 9730 4736 2856 6672 5476 11,988 68,116 5772 17,834 11,692 539,097 11,690 13,056 12,580 8 AVERAGE DEP TH <u>ئ</u>ـ CHANNEL DIMENSIONS (2) 10 10 **BOTTOM** WIDTH (9) 00,00 4 8 27 8 4 10 12 14 WIDTH T0 P Ft. (5) 14 118 22 26 13 14 13 15 16 20 22 24 13 13 13 13 DISCHARGE c. f.s. (4) 144 264 274 73 22 17 17 28 58 75 18 64 23 79 58 106 174 229 44 70 WATERSHED 2368 188 840 284 1268 1596 2666 1180 1592 2472 356 800 400 624 672 1912 2560 4934 5368 268 200 368 868 1196 676 748 1060 1792 3236 4524 1080 980 784 368 Ac. (3) 8100 5500 172,500 LEN GTH 2400 3100 5900 4700 8700 7400 1700 8100 6400 3500 3200 7200 7900 2100 3700 4600 7000 5700 4800 1500 5600 31,300 8500 2900 6400 4400 6700 6000 2000 4500 3900 5600 6700 1500 Ft. Total-6 Total-7 L-10 L-11 L-12 L-12 L-12 L-13 L-13 L-13 L-13 L-13 L-13 L-13 L-13 L-14 L-16 L-17 CANAL L-3 6-7 6-7 L-5 L-5 6-7 . . . L-2 L-2 1-6 0 0 X X

Sheet 3 of 4 41,682.00 30,714.00 4528.00 35,743.00 2412.00 10,243.00 ESTIMATED Dollars TOTAL (13) COST 2-15' R.C.Brs. BRIDGES - NEW Length & Size Br. 30' R.C. Br. R.C. Br CULVERTS & - 60" - 49" - 60" 80' - 24" - 18" - 24" - 30" - 24" 36" .. 69 -- 60 " - 42" U.T. (12)30. 50 , 30. 30 401 60. 30 30, 401 40 15' 15' Length & Size CULVERTS LOWERING - 18" 1 1 1 1 1 1 1 900 Area 1 - Hanahan - Goose Creek - Carnes Crossroads - Oakley RT. OF WAY REQUIRED WIDTH Ft. (10) 38 38 38 88 38 41 49 57 38 38 38 38 41 38 38 38 38 38 38 38 88 RT. OF WAY CLEARING Ac. (9) 5.6 2.0 3.8 3.9 1.8 0.7 0.7 4.8 63.2 63.0 6.4 6.2 2.2 EXCAVATION Cu. Yds. 11,988 6068 10,915 4440 7844 3552 1480 3996 6364 9768 5344 3256 6956 5916 19,500 5032 2368 7696 5328 3848 4292 132,710 11,248 6012 17,760 15,290 7844 12,580 80,502 4884 20,461 4884 12,773 3848 29,793 4440 6660 10,228 105,484 AVERAGE Ft. (7) DEP TH CHANNEL DINENSIONS 01 01 01 01 01 01 0 0 0 0 0 0 0 0 10 10 2 2 2 2 2 10 10 n BO TTOM WIDTH (9) m 4 8 4 7 01 8 8 m m ∞ m mന ന m TOP WIDTH Ft. (5) 13 13 13 13 13 13 13 13 13 13 13 13 14 13 13 13 13 13 13 DISCHARGE c. f. s. (4) 52 98 156 224 44 30 52 40 72 1122 253 399 24 48 48 24 57 101 59 43 26 73 20 20 35 17 17 16 29 16 16 103 44 80 177 32 51 33 WATERSHED 308 240 470 200 384 184 1032 140 240 760 1636 2864 4352 624 392 384 760 924 2192 3820 136 304 128 352 376 736 736 200 192 352 190 244 492 Ac. (3) 4400 5300 2400 1000 2700 6600 4300 LEN GTH 5900 7900 49.000 7600 3600 8000 5500 6600 5300 8500 3300 3200 3300 3000 2900 5900 4500 45,100 6500 2200 2900 3600 3900 3400 1600 4100 5200 3600 5300 3000 16,800 Ft. (2) Total-10 Total-14 Total-11 Potal-12 Total-13 Total-8 Total-9 M - 13M - 10M - 10M-11 M-11 M-11 M-11 M-12 M-12 M-12 M - 13CANAL . . . L-2 L-3 L-6 L-7 L-8 M - M M-9 M-9 1-2 L-3 L-5 L-4

Sheet 4 of 4 6575.00 2595.00 2590.00 433,218,00 ESTIMATED 17,411.00 Dollars TO TAL COST (13) BRIDGES - NEW Length & Size CULVERTS & Br 42" 36 (12) 15' R.C. 1-1 | | 1 1 30 30 CULVERTS LOWERING Length & Size $\widehat{\Xi}$ 1 | 1 1 Area 1 · Hanahan · Goose Creek · Carnes Crossroads · Oakley REQUIRED RT. OF WAY WIDTH Ft. (10) 38 41 38 38 38 38 94 RT. OF WAY CL EARING adequate adequate (6) 22.9 3.5 9.0 0.5 0.9 3.8 562.4 Ac. considered is considered EXCAVATION 5920 5678 5476 17.074 1036 1,301.578 Cu. Yds. 6956 6956 7252 7252 1776 6444 29,600 38,856 (8) is BOTTOM AVERAGE as constructed as constructed D EP TH Ŧ, (7) CHANNEL DIMENSIONS 20 20 N N 2 2 S S WIDTH £ţ. (9) 1 60 44 60 35 m $^{\circ}$ ന ന canal Present canal TOPWIDTH Ť, (5) 13 $\stackrel{\square}{\mapsto}$ 13 45 13 29 35 Present DISCHARGE c. f. s. (4) 328 93 44 61 WATERSHED 162 230 2190 1304 296 672 208 Ac. (3) 276 404 124 150 4000 3400 3700 11,100 4700 4900 700 4000 1200 7100 581,100 LEN GTH Ft. (2) Total-16 Total-18 Total-15 Total-17 M-15 M-15 M-17M-18 M-18 M-18 M-18 M-18 CANAL M - 16Area 1 L-1 . . . Grand Total

Area 2 - Moncks Corner - Oakley

30,195.00 3031.00 42.041.00 17,229.00 ESTIMATED Sheet 1 of Dollars COST (13) BRIDGES - NEW Length & Size R.C.Brs. 30' R.C. Br. 15' R.C. Br. R.C. Br. 30' R.C. Br. CULVERTS & - 54" - 54" - 24" - 60" - 36" - 18" - 36" - 54" - 36" - 60" - 48" - 30" - 24" (12)180 201 40 30 404 601 30 30 30 40 1001 2-15' 15 LOWERING Length & Size 54" CULVERTS 24" Ξ 1 404 404 REQUIRED RT. OF WAY WIDTH Ft. 38 38 38 CLEARING adequate 1.8 9.3 9.3 1.2 1.2 4.4 1.2 (6) 2.0 2.8 4.6 8.9 8 8 8 8 2.2 1.2 5.9 1,9 37.0 3.5 3, 5 0.4 0.5 EXCAVATION consider 44440 3700 2220 3700 6120 21,208 21,420 3108 5328 41,687 1332 9176 5100 6266 20,572 3330 13,900 2812 1036 2664 2516 2368 4736 5328 6956 5920 4440 5920 4440 2516 8880 2368 1924 4292 925 5624 4144 6364 Cu. Yds. 81,604 7104 111,858 (8) AVERAGE <u>۔</u> (7) CHANNEL DINENSIONS 10 BO TTOM WIDTH (9) 03 00 60 00 \mathfrak{O} 9 0 0 0 0 1 cana WIDTH Ft. (5) 13 13 13 13 14 13 13 13 13 13 13 13 113 113 113 113 113 113 113 13 13 13 13 13 13 13 13 13 DISCHARGE c. f.s. (4) 23 31 55 83 154 1154 111 111 128 288 15 28 54 107 1118 211 15 28 64 15 15 20 27 43 56 131 231 31 38 25 37 29 11 48 78 22 20 20 36 31 WATERSHED 820 3989 1977 352 409 320 400 1332 3456 4932 120 224 708 1236 280 240 488 420 996 256 572 662 1200 60 116 306 56 110 600 521 144 Ac. (3) 40,800 6000 1600 1300 2900 3600 4700 4000 3000 3000 1800 5000 1900 LEN GTH 2500 1500 2500 8800 6800 1600 3200 4700 3800 6200 2600 3900 3800 4000 500 700 1800 2100 3600 3000 1700 2500 7400 4300 4800 23,200 Ft. (2) Total-2 Total-3 Total-4 Total-CANAL L-5 L-5 L-6 M-1M-2 L-6M-4No. M-1L-1

ENGINEERING AND DESIGN DATA Area 2 - Moncks Corner - Oakley

Sheet 2 of 3 21,312.00 TOTAL ESTIMATED 40,334.00 10,986.00 4469.00 4344.00 Dollars CO ST (13) 15' W.T. Br. 15' R.C. Br. 2-15' R.C. Brs. BRIDGES - NEW Length & Size 15' R.C. Br. Br 40' - 42" 60' - 36" - 36" 24 CULVERTS & 40' - 24" 40' - 54" - 54" - 48" - 18" 48 - 48" 15' U.T. ī 30' 401 30. 60 30 40 Length & Size 60' - 18" 60' - 24" - 24" - 36" CULVERTS LOWERING - 24" - 36" - 15" Ξ 1 1 80 80, 204 80 40 REQUIRED RT. OF WAY WIDTH Ft. 38 38 46 38 38 38 38 38 38 38 38 38 38 38 38 38 38 38 RT. OF WAY CL EARING 1.5 4.5 2.3 8.9 9.9 2.5 2.9 3.0 0.0 8.9 5.9 4.9 4.0 48.8 6) 9°9 2.6 EXCAVATION Cu. Yds. 2960 9996 4440 24,204 4588 5180 9768 4440 3150 3256 6956 3700 1036 6012 6512 5624 2960 4884 8880 7752 8194 3885 2780 2780 12,488 12,488 10,224 17,131 6660 5328 2220 8732 2516 2664 3552 3848 112,988 13,468 12,284 65,434 AVERAGE DEP TH <u>ئ</u> CHANNEL DINENSIONS (7) 10 10 10 10 10 N N 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 BOTTOM WIDTH į. (9) n ന ന ന ന ന 10 110 110 116 116 20 4 2 0 0 0 0 0 0 0 WIDTH T0 P ئ<u>ہ</u> ڪا (8) 13 13 13 13 13 13 13 13 13 13 13 13 13 13 13 16 DISCHARGE c. f. s. **(*)** 22 31 68 94 158 203 43 43 111 23 61 111 26 36 23 57 76 205 219 2228 2228 2321 3321 432 432 432 73 73 52 73 WATERSHED 1248 1352 1480 2136 2408 240 260 756 240 88 268 376 1048 1412 3008 368 1268 1684 48 128 380 52 88 104 444 128 294 460 676 Ac. 120 3000 3100 3300 1500 4900 6600 8300 1500 1000 3700 3600 3900 4700 3600 3800 6000 3400 1000 3200 3200 2400 4500 700 2900 5200 5900 1800 4400 2400 45,900 1700 14,500 38,400 LEN GTH Ft. M-5 L-1 Total-5 Total-6 Total-7 Total-8 Total-9 CANAL ₩°. M-7 L-1 L-2 L-3 L-3 M - 5 9-W M-6 M-7 φ Ψ M M M M-8 M-8 8-M 8-M 8-M M-9 M-5 M-7 M-8 M-8 $\mathbb{A}_{-\beta}$ M-8 M-8 M-8 M-9 1-2 M-9 L-2

ENGINEERING AND DESIGN DATA Area 2 · Moncks Corner · Oakley

Sheet 3 of 3	TO TAL	ESTIMATED COST	Dollars (13)	5572.00	179, 513.00
		CULVERTS & BRIDGES - NEW	Length & Size (12)	60' - 54"	
		CULVERTS	Length & Size (11)	111	
	REQUIRED	RT. OF WAY WIDTH	Ft. (10)	38 38 38	
r - Oakley		∦ΑΥ N G	Ac. (9)	20.00	8
Area 2 - Moncks Corner - Oakley		_	Cu. Yds. (8)	5920 4588 3404 13,912	480,695
Z - Mor	SIONS	AVERAGE DEP TH	Ft.	מממ	
Area	25 1		/Ft. (6)	ппп	
	CHAND	TOP WIDTH	Ft. (5)	13 13 13	
		DISCHARGE	c. f.s. (4)	20 35 46	
		WATERSHED	Ac. (3)	252 472 660	
		LEN GTH	Ft. (2)	4000 3100 2300 9400	258,300
		CANAL	No.	M-10 M-10 M-10 Total-10	Area 2 Grand Total

Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon

### CHANNEL DINKENSIONS ### Ft. Ft. Cu. Yde. 55				*****							_	
1,504,846 10				CHANN	EL DIMEN	SIONS			REQUIRED			TOTAL
Color Colo	¥ ¥	TER SHED	DISCHARGE	TOP WIDTH	BO T T O M W I D T H	AVERAGE DEP TH	EXCAVATION	OF WAY	RT. OF WAY WIDTH	CULVERTS	TS &	ESTIMATED COST
15		Ac. (3)	c. f. s. (4)	Ft. (5)	Ft. (6)	Ft. (7)	Cu. Yds. (8)		Ft. (10)		_	Dollars (13)
140	-	832	56	C 1		10 1	8732	4.3	38	1	R.C.	
205		1208 2484	041	C T 6		O 10	0473	T 0	4 r.	1	-	
244 26 16 5 31.120 12.3 79 79 70 77.7 71.1 17.1 17.1 18.2 2.4 4 4 4.15.964 6.7 7 82 7 7 9 7 9 7 7 9 7 9 7 9 7 9 7 9 7 9 9 7 9 9 7 9		3996	206	4 53	14	υ rc	11,616	4.7	80		. G	
171 28 18 18 18 18 18 18 1		4884	244	26	16	70	31,120	12.3	73		C.T.	
178 32 24 4 4150 417 418 4		5228	257	28	18	Ω,	12,780	2.0	78	!	-	
10		18,838	171	N 0	23 0	4.	4150	1.7	& & &	-	C.T.	
203		20,294	181	2 K	26	4 4	9343 15,984	0.40	0 00		: : :	
293 46 40 40 10.300 6.5 1199		23,154	203	36	28	4	22,752	6.0	16	1		
394 58 45 4 15,972 6.3 130 130 144 68 60 4 41,712 16.1 165 165 165 165 165 165 165 165 165 165 165 165 165 165 165 165 165 16.1 165		36,084	293	48	40	4	16,300	6.5	119	-	1	
1846 186		37,432	304	53	45	4	15,972	6.3	130	!	1	
414 68 60 4 4 17.72 16.1 165 42" 426 68 60 4 4 25.56 6.9 165 42" Present canal as constructed is considered adequate Present canal as constructed is considered adequate Fresh canal as constructed is considered adequate 65 14 4 4 5 12.050 6.9 1.6 5 12.050 125 20 10 5 12.050 7.9 1.6 5 7 15 8.7 125 20 10 5 12.050 8.8 18 8 78 8.8 270 28 18 5 12.050 8.8 18 78 8.8 280 20 10 5 20.368 8.8 18 8 78 8.8 280 20 20 20 20 30.0 0.7 280 30 20 20 30.0 0.7 280 30 20 30 30 0.7 280 30 20 30 30 0.7 280 30 20 30 30 0.7 280 30 20 30 30 0.7 280 30 20 30 30 0.7 280 30 20 30 30 0.7 280 30 30 0.7 280 30 0.0 0.7 280 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 0.0 280 0.0 0.0 0.0 280 0.0 0.0 0.0 280 0.0 0.0 0.0 280 0.0 0.0 0.0 280 0.0 0.0 0.0 280 0.0 0.0 0.0 280 0.0 0.0 0.0 280 0.0 0.0 0.0 280 0.0 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280 0.0 0.0 280		46,832	366	28	20	4	55,743	18.4	163	1	1	
19		53,156	407	8 0	09	4 4	41,712	16.1	165	1	1	
Present canal as constructed is considered adequate 125		76.760	4 1 4	D 02	0 0	7' \	20,090	ກ ແ	165 165	!		
Present canal as constructed is considered adequate Fresent canal as constructed is considered adequate 5 Fresent canal as constructed is considered adequate 5 Fresent canal as constructed is considered adequate 5 Fresent canal as constructed is considered adequate Fresent canal as construct		316	12 CV	n (1) m	។ ភេ	9916	. 4 	0 00		1	
Present canal as constructed is considered adequate 65		708	Presen			tructed j		adequat)		1	
Present canal as constructed is considered adequate 125		1132	Presen		20	tructed i		adequat				
125		328	Presen		9	tructed i		adequat		1 1		
125 18		984	65	14	4	ις	4509	2.2	41		- 48"	
155 20 10 5 11.398 4.8 57 15 R.C. 229 26 16 5 2892 1.6 57 250 28 18 5 22.578 8.8 78 270 28 18 5 22.578 8.8 78 280 20 20 20 20 20 20 281 30 20 20 20 20 20 282 28 28 28 28 20 20 283 28 28 28 20 20 20 356 34 24 5 20.943 17.1 99 380 2.7 36 28 29 20 20 381 38 28 29 20 382 393 20 20 383 20 20 20 384 20 20 385 20 20 20 386 20 20 20 387 20 20 388 20 20 388 20 390 20 300 20 300 20 300 20 300 20 300 20 300		2164	125	18	00	7.	12,050	о С	0,0	!	·	
155 20 10 5 3892 1.6 57 73 800 C.T. 229 26 16 5 22.578 8.8 78 800 C.T. 250 28 18 5 10.650 4.1 78 800 C.T. 288 30 20 5 8334 3.2 844 800 C.T. 288 30 20 5 8334 3.2 844 800 C.T. 356 34 24 5 20.943 7.9 94 800 C.T. 360 26 5 20.943 7.9 94 800 C.T. 416 38 28 5 20.943 7.9 94 800 C.T. 583 50 40 5 30.652 13.2 10.5 56 13 3 5 10.212 5.1 38 56 13 3 5 10.212 5.1 38 57 10.212 5.9 46 583 50 40 5 30.654 46 4.0 590 15 C.T. 50 44 13 3 5 8140 4.0 50 13 3 5 10.528 2.6 38 50 14 5 16.544 6.7 68 50 15 C.T. 50 17 C.T. 50 18 C.T. 50 19 C.T. 50 10.544 6.7 6.7 50 10.545 10.544 6.7 50 10.545 10.545 10.545 50 10.545 10.545 10.545 50 10.545 10.545 10.545 50 10.545 10.545 10.545 50 10.555 10.555 10.555 5		2800	155	20	10	ιO	11,398	4.8	57	1	R.C.	
229 26 16 5 3890 1.5 73 30'C.T. 252 28 18 5 22.578 8.8 78 30'C.T. 268 30 20 20 5 8334 3.2 84 30'C.T. 268 34 24 5 20.943 7.9 94 30'C.T. 356 34 24 5 20.943 7.9 94 30'C.T. 3571 36 26 5 20.943 7.9 94 30'C.T. 358 13 3 5 10.212 5.1 38 155 105 155 105 155 105 105 155 105 105 155 105 105 155 105 105 155 105 105 155 105 105 155 105 105 155 105 105 155 105 105 155 105 105 155 105 105 105 105 105 105 105 105 105		2832	155	20	10	10	3892	1.6	57	-	1	
252 28 18 5 22.578 8.8 78 30' C.T. 288 38 2 22.578 8.8 78 30' C.T. 288 38 2 20.943 3.2 844 30' C.T. 288 38 2 20.943 7.9 94 30' C.T. 288 38 2 20.943 7.9 94 30' C.T. 30' C.T. 36 20' 848 11.1 99 30' C.T. 30' C.T. 36' 652 13.05		4512	229	26	16	ſΩ	3890	1.5	73	1	C.T.	
288		5044	252	28	18	ıΩ	22,578	8.0	78	-	C.T.	
288 30 20 5 8334 3.2 84 356 34 24 25 5 20.943 7.9 94 371 36 28 5 14.664 5.5 1055 583 50 40 5 36.652 13.2 137 56 13 3 5 10.212 5.1 38 Present canal as constructed is considered adequate Present canal as constructed is considered adequate Present canal as constructed is considered adequate Present canal as constructed is considered adequate Present canal as constructed is considered adequate Present canal as constructed is considered adequate Present canal as constructed is considered adequate Present canal as constructed is considered adequate Present canal can		5476	270	28	18	Ŋ	10,650	4.1		1	C. I.	
356 34 24 5 20.943 7.9 94		5940	288	30	50	S	8334	3.8	84	-	1	
36 26 29 11:1 99		7724	356	9,4	24	ıo ı	20,943	7.9	90		1	
583 50 40 5 36.652 13.2 100 583 13 3 50 40 5 36.652 13.2 137 590 16 6 5 10,212 5.9 46 70		8180	37.1	900	:O C	O F	29,848	11.1	D (1		
13 3 5 10,212 5.9 46 15 C.T. Present canal as constructed is considered adequate 15 C.T. Present canal as constructed is considered adequate 15 C.T. Present canal as constructed is considered adequate 15 C.T. Present canal as constructed is considered adequate 24 Present canal as constructed is considered adequate 24 Present canal as constructed is considered adequate 24 Present canal as constructed is considered adequate Present canal as constructed is considered adequate Present canal as constructed is considered adequate Present canal as constructed is considered adequate Present canal as constructed is considered adequate		0 0 TO 0	4 L C C C	D C	0 0	O FU	14,004 26,650	0.0,0	100	1	1	
Present canal as constructed is considered adequate 213 24 14 5 16.544 6.7		B40	. rc	÷ €) M) IC	10.818	. rc	` aa		- E	
Present canal as constructed is considered adequate 213 24 14 5 16.544 6 6.7		1456	06	16	0	10	13,056	0.00	46			
Present canal as constructed is considered adequate 2.6 38 15' C.T.		1440	Preser			tructed		adequat		1	1	
44 13 3 5 5328 2.6 38 15 C.T. 50 13 3 5 8140 4.0 38 15 C.T. 60 13 3 5 8140 4.0 38 15 C.T. 70 Present canal as constructed is considered adequate 24' - 48' 71 24 14 5 16.544 6.7 68 72 73 24 14 5 16.544 6.7 73 74 75 16.544 6.7 68 74 75 75 75 75 75 75 75		1552	Preser		8	tructed					C.T.	
Present canal as constructed is considered adequate 213 24 14 5 16,544 6.7 68 Present canal as constructed is considered adequate 226 227 248 249 259 260 270 280 281 281 282 283 284 284 285 285 285 286 286 287 287 288 288 288 288 288 288 288 289 289 280		620	44	13	m	Ω	5328		38	1	C. II.	
Present canal as constructed is considered adequate Present canal as constructed is considered adequate Present canal as constructed is considered adequate 213		920	09	13	m	7.5	8140	4.0	38	1		
Present canal as constructed is considered adequate Present canal as constructed is considered adequate Present canal as constructed is considered adequate 213 24 14 5 16.544 $\overline{6.7}$ 68		168	Presen				S			-	-	
Present canal as constructed is considered adequate Present canal as constructed is considered adequate 213 24 14 5 16,544 6.7 Present canal as constructed is considered adequate		448	Presen		S	tructed i				1	- 48	
Present canal as constructed is considered adequate 213 24 14 5 16.544 $\overline{6.7}$ Present canal as constructed is considered adequate		1088	Presen		9 S					-	-	
Present canal as constructed is considered adequate		2970	Presen		S	tructed ;				-		
Present canal as constructed is considered		4134	213		14	ro	16,544		68	-	!	
THE THE PERSON ASSESSED. THE PERSON ASSESSED.		872	Presen		as cons		co.			1	1	
Fresent canal as constructed is considered		1802	Present	t canal	as cons		s considered	l adequate		1		

ENGINEERING AND DESIGN DATA

Sheet 2 of 6 TOTAL ESTIMATED Dollars (13) COST Br. Br. Br. Br. Br. Br. Br. Length & Size BRIDGES - NEW Br, Br. CULVERTS & - 48" - 54" - 48" C.T. C.T. C. T. C.T. C.T. E E E (12) 201 20. 15, 15' 45' 20 45' 15 15, 45' 30 30 30 ' CULVERTS LOWERING Length & Size Ξ Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon RT. OF WAY REQUIRED WIDTH Ft. (10) 105 124 124 137 137 88 89 99 00 00 38 44 44 44 44 46 46 46 84 49 38 46 44 constructed is considered adequate is considered adequate is considered adequate is considered adequate RT. OF WAY CLEARING (6) 8.2 4.3 3.2 23.3 1.5 1.5 5.7 17.9 EXCAVATION 21,996 5772 2405 10,360 31,654 12,950 4440 2960 9028 9723 48,216 22,654 6216 8880 7140 5180 15,540 2812 9472 9768 11,398 6364 4144 6660 5344 3241 30,000 9881 Cu. Yds. 11,670 30,943 13,260 13,320 11,022 10,730 12,448 15,865 48,152 10,906 16,497 15,827 (8) constructed constructed constructed BOTTOM AVERAGE DEP TH (2 CHANNEL DIMENSIONS 8 28 сд го 8 WIDTH (9) canal canal 4000000 16 7 canal canal WIDTH Present Present Present Present (2) £ £ TOP 45 32 36 38 13 DISCHARGE c. f.s. (4) 509 515 544 328 370 405 141 234 340 549 145 20 77 1110 1119 46 1109 1155 53 25 25 25 25 25 25 27 273 49 77 108 392 397 98 WATERSHED 11,982 1836 7000 980 228 1216 4672 8560 8760 12,658 1600 1224 1848 2056 656 1148 1200 8032 928 1360 700 736 2268 2512 1816 5892 8384 12,778 2608 252 780 320 1008 4764 3148 6304 8944 11,642 880 Ac. 8400 3900 1300 4800 6500 6000 2000 6400 4400 4300 2800 4500 2000 9500 9400 2400 3500 3000 3900 1400 700 3800 1900 6000 1900 7000 3200 5800 6000 10,400 LEN GTH Ft. (2) L-15 L-16 L-16 L-16 L-16 L-17L-17 L-18 L-20 L-23 L-23 L-13 L - 13L-13 L-14 L-14 L-14 L-14 L-15L-19 L-21 L-22 L-22 L-23 L-23 L-23 L-23 L-24 L-11 L-12 L - 13L-14 L - 14L-25 L-26 L-26 CANAL L-14 L-14 L-14 . . .

ENGINEERING AND DESIGN DATA

ESTIMATED 439,698.00 Sheet 3 of Dollars TOTAL COST (13) BRIDGES - NEW Length & Size 15' C.T. Br. 15' C.T. Br. C.T. Br. C.T. Br. Br. Br. 30' C.T. Br. 15' C.T. Br. CULVERTS & 30' - 48" - 60" 30' - 48" - 60" 30' - 42" - 48" - 60" - 48" - 30" C. T. 30' C.T. C. T. 15' C.T. (12) 301 30 40. 30 404 40, 15' 15' 15' 5 LOWERING Length & Size CULVERTS 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon REQUIRED RT. OF WAY Ft. 38 38 38 38 38 38 38 38 38 38 38 449 55 57 57 57 57 57 38 RT. OF WAY CLEARING adequate 1.1 Ac. (9) considered EXCAVATION 10,008 11,676 5180 5476 4676 4995 8288 20,372 6882 12,780 10,656 3848 4676 13,024 17,248 4144 4144 8140 5328 6475 Cu. Yds. 5180 22,172 14,175 6300 6993 5476 3108 10,545 6956 3256 4736 8584 1,353,255 13,206 8517 (8) AVERAGE constructed D EP TH ٠ ٢ (7) CHANNEL DIMENSIONS BO TTOM WIDTH (9) ന ന WIDTH TOP : ۲ (2) Present Area DISCHARGE c. f.s. (4) 143 155 207 222 368 381 WATERSHED 216 240 1328 1068 5668 6056 4678 7846 8266 1052 396 1144 1344 3024 3688 292 1148 2548 2832 4016 4304 216 Ac. (3) 5600 2800 2800 5700 4300 3100 4200 2800 2700 3500 7200 2600 3700 4400 5000 3700 2100 2200 2800 3200 4700 2200 3200 5100 3200 3600 1500 3000 3100 1500 3000 5500 5800 446,400 L EN GTH Ft. (2) L-31 L-31 L-31 L-32 L-33 L-33 L-34 CANAL L-34 Total-1 L-27 L-28 L-29 L-30 L-31 L-31 L-31 L-35 L-38 L-38 L-34 L-34 L-36 L-36 . . .

- 30 -

Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon

Sheet 4 of 6	TOTAL	ESTIMATED COST	Dollars (13)										41,561.00		7 7	4000.00			8112.00												,														0	52.097.00
		CULVERTS & BRIDGES - NEW	s (15' C.T. Br.	1	ļ.	1	30' - 48"	ì	30' - 48"	}	-	ļ	30' 54"			30' - 60"	-	!		30' II T. Br.	• + •	-			ł	30' - 42"	15' U.T. Br.			-	- 1	-1	20' - 48"	15' U.T. Br.		15' H T Br.	• • •	-	!	40' - 36"	-	20' - 48"	1	1	
рапоп		CULVERTS	Length & Size (11)		-	-	-	!	1		!	-	-		1		!	!	!		1 1			-	-	-	-	1			1	-		!	!	1	1 1		!	-	-	-	40' - 18"	-		
Hope · Lebanon	REQUIRED	RT. OF WAY WIDTH	Ft. (10)	38	38	44	46	38	38	38	38	00 00		38	33		38	® (0 €	D D		94	80	89	89	73	73	38	38	88	0 to	52	38		38	8 6	D 0	m c	0 00	0 00	38	38	38	38	38	38	
Store - INew I		RT. OF WAY	Ac. (9)	3.3	3.7	2.7	6.8	ත _: ද	8.0	2.4	1.7	23	adequate 57.6	2.9	83 m	0.0	2.7	4.7	12.0	4	auequare 3.1	2.0	1.0	5.8	1.5	11.5	4.2	6. 8	0		4.	1.7		1.4	ນ (ນໍ (, v, v	m o	0000	0 0	80.00	7.0	2.0	2.7	2.3	25 :51	69.5
- 1		EXCAVATION	Cu. Yds. (8)	6660	7400	5735	6528	5772	1628	4884	3404		s considered 128.816	5920	5328	TT, 640	5476	9472	24.272	700		9135	2464	14,432	3890	29,175	8436	5920	4000	3133	3133	3404		2812	0999	0.000	3700	1776	3996	5624	1480	3996	11,544	4588	5032	153.254
assamassaw - Cooper s	SIONS	AVERAGE DEP TH	Ft. (7)	22	73	D	ιO I	ιΩ	10	10	ω I		constructed is	22	ıΩ		ıΩ	Ωп	ი	100+0000+0000	-	ν ιο	20	22	Ω	Ω	D	20	ıc	0 10	10	10		TO I	Ωı	ΩШ	O 10) ro	10	r0	τO	10	го	ıΩ	22	_
assamas	NEL DINENSIONS	BO TTO M WIDTH	Ft. (6)	3	က	5	0	m	m	ന	m		as cons	n	m		က	m (2		4 U) C	14	14	16	16	n	m	cr	o 00	00	က		n 1	n (m (m (m	o m) m	n	n	m	m	က	m	
٠ . ×	CHANNEL	TOP W1DTH	Ft. (5)	13	13	15	16	13	13	133	13		nt cana.	13	13		13	e	۳ ۲	+		0 03 1 03	24	24	26	26	13	13	Ç	19 6	18	13		13	ກ ເ ⊢ ⊢	უ r	_ ⊢ ກ ເ	n en	n ∈	13	13	13	13	13	,13	
Area		DISCHARGE	c. f.s. (4)	48	73	12.2	131	30	52	27	11	25	Present	27	40		35	22	2/	0 2 0 2 0	148	262	282	888	329	343	32	47	Γ.	131	132	28		38	020	3.7	/ 4 r	υ, -(□	1 60	88	80	17	25	44	13	
	-	WATERSHED	Ac. (3)	089	1140	2104	2832	400	756	344	120	316	612	360	552		472	760	227	0 0 0	2708	5246	5736	6172	0069	7312	420	999	780	22.96	2346	.368		528	724	212	0,7%	742	088	440	88	196	328	668	140	
		LEN GTH	Ft. (2)	4500	2000	3100	3200	3900	1100	3300	2300	3000	60,500	4000	3600	000/	3700	6400	16.400		3400	2900	400	4100	1000	7500	2700	4000	0000	1300	1300	2300		1900	4500	4000	1300	1200	2700	3800	1000	2700	7800	3100	3400	76.900
		CANAL	. O. V	L-3	L-3	L-3	L-3	L-4	1-4	L-5	L-6	I-7	L-7 Total-2	M-3	M-3	Total-3	M-4	M-4	M-4 Total-4	2	O 10	Σ 	M-5	M-5	M-5	M-5	1-1	1-1	+	1 -	H H	1-2		L-2	C .	۳ ا ا	n 0	o e:	, i	1-4	1-5	L-5	L-6	1-6		Total-5

Area 3 - Wassamassaw - Cooper's Store - New Hope - Lebanon

Column C					CHANN	FI DIMEN	SHOIS						
Color Colo	ANAL	LENGTH	WATERSHED	DISCHARGE	TOP	BOTTOM	AVERAGE	EXCAVATION	OF WAY	REQUIRED RT. OF WAY		S	ESTIMATED
Figure F	No.	Ft. (2)	A C.	c. f. s. (4)	Ft. (5)	WIDTH Ft. (6)	DEP TH F t. (7)	Cu. Yds. (8)	CLEARING Ac. (9)	WIDTH Ft. (10)	RING & Siz 1)	. 00	COST Dollars (13)
1,000 1,004 1,004 1,004 1,004 1,004 1,004 1,004 1,004 1,004 1,004 1,004 1,004 1,004 1,004 1,004 1,004 1,004 1,004 1,00	M-6 tal-6	5700	248	20	13	1	ω	8436 8436	4.2	38	1	1	2863.00
1,000 1,70	M-7	3200	264	21	13	m	5	4736	2.5	38		- 48	
1,000 1,00	M-7	7100	944	53	13	က	D.	10,508	5.2	38	-	R. C.	
19,000 10,000 1	M-7	5100	1036	67	13	m (ro n	7548	т. С. т	38	1	ъ. С	
Color Colo	M-7 tal-7	22,400	77.51	ò	۳ ۲	n	n	33,152	16.4	D N		1	13,326.00
1,000 10,1	M-8	2900	292	23	13	e	5	4292	2.1	38	ŀ	1	
1100 1108 776 13 3 5 1440 0.4 39 15 15 15 15 15 15 1	M-8	0009	612	43	13	m	D	8880	4.4	38	-	U.T.	
100 252 17	W-W	5700	1188	76	13	en (ro r	8436	4.2	38	1	R.C.	
10	W-0	1100	1228	777	m 4	ന	ro n	1628	o ,	33		;	
12.00 1.00	0 00 	3000	2010 2056	162	17	0 1	o ro	2448 6660	1.1 0.0	94 4	1 1		
1200 1200 1200 120 1	W-8	5300	3248	175	17	7	· Ω	11,766	υ	49		1	
1200 1286 75 13 3 5 6466 2.7 38 15 0.7. Br. 24 39 15 0.7. Br. 25 35 35 35 35 35 35 35	L-1	3500	540	34	13	က	D.	5180	2.6	38		-	
11 12 13 14 15 15 15 16 15 16 15 16 15 16 15 16 15 15	L-1	3700	768	52	13	က	D	5476	2.7	38	-		
1,500 1,50	L-1	0000	1128	72	13	თ ი	ហ ៣	8880	4.0	8 0	-		
3300 544 39 13 3 5 4884 2.4 38 15' C.T. Br. 4700 948 62 13 3 5 5476 3.5 38 15' C.T. Br. 4700 1730 103 14 4 5 1169 0.6 44 15' C.T. Br. 4500 2506 141 16 15 5 10.608 4.6 44 15' C.T. Br. 4500 2504 143 16 6 5 10.608 4.9 4.6 15' C.T. Br. 4500 2504 143 15 15 10.608 4.9 4.6 15' C.T. Br. 4500 2504 153 16 6 5 10.608 4.9 4.0 15' C.T. Br. 4500 2504 2504 153 13 3 5 20.730 30.0 38 15' C.T. Br. 5700 2504 2504 2504 2.1 2.1 2.1 2.1 2.1 5700 2505 2504 2.1 2.1 2.1 2.1 2.1 2.1 5700 2506 2504 2.1 2.1 2.1 2.1 2.1 2.1 5700 2506 2504 2.1 2.1 2.1 2.1 2.1 2.1 5700 2506 2504 2.1 2.1 2.1 2.1 2.1 5700 2506 2504 2.1 2.1 2.1 2.1 2.1 5700 2506 2.1 2.1 2.1 2.1 2.1 2.1 5700 2506 2.1 2.1 2.1 2.1 2.1 2.1 5700 2506 2.1 2.1 2.1 2.1 2.1 2.1 5700 2506 2.1 2.1 2.1 2.1 2.1 2.1 5700 2506 2.1 2.1 2.1 2.1 2.1 2.1 5700 2.1 2.1 2.1 2.1 2.1 2.1 2.1 5700 2.1 2.1 2.1 2.1 2.1 2.1 2.1 5700 2.1 2.1 2.1 2.1 2.1 2.1 2.1 5700 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 5700 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 5700 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 5700 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 5700 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 5700 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 5700 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 5700 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 5700 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 2.1 5700 5700 5700 5700 5700 5.1 5.1 5.1 5.1 5.1 5.1 5.1 5.1 5.1 5.1 5.1 5.1 5.1 5.1 5.1 5	L-1 tal-8	41,500	1,500	5	C T	n)	68,234	32.8	0	 	1	24,189.00
3700 764 52 13 5 5476 2.7 38 15 C.T. Br. 700 1730 103 14 4 5 1109 0.6 41 15 C.T. Br. 700 1730 104 4 4 4 15 C.T. Br. 5300 1750 141 16 6 5 10.608 4.6 44 15 U.T. Br. 5500 2794 153 13 5 10.608 4.6 46 15 U.T. Br. 5500 2794 13 3 5 10.608 4.6 15 U.T. Br. 2500 2794 3 5 7944 3.0 15 U.T. Br. 2500 288 23 13 3 5 7992 4.0 39 15 0.7 1.8 1.0 1.8 1.8	4-9	3300	544	39	13	m	2	4884	2.4	38	-	C.T.	
4700 948 62 13 3 5 6956 3.5 38 15 C.T. Br. 5300 1966 116 16 5 9905 4.6 15 0.7 18. 4500 2506 116 16 6 5 10.060 4.6 15 10.10. Br. 4600 480 35 13 3 5 6008 3.4 46 15.0 II. Br. 15 5700 480 35 13 3 5 6008 3.4 46 15.0 II. Br. 15 5700 480 46 13 3 5 7992 4.0 38 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0	6-1	3700	764	52	13	က	22	5476	2.7	38	-	C.I.	
5700 1730 103 14 4 5 9180 4.6 44 157 C.T. Br. 4500 2506 141 16 6 5 9180 4.1 46 15.0.T. Br. 5200 2794 153 13 3 5 10.608 3.4 39 15.0.T. Br. 5200 752 13 3 5 7944 3.9 38 15.0.T. Br. 5200 752 13 3 5 7944 3.9 38 15.0.T. Br. 2500 13 3 5 7942 4.0 38 30'54" 22 5400 652 46 13 3 5 7922 4.0 38 30'54" 22 5400 652 13 3 5 7922 2.1 38 30'54" 790 13 3<	6-1	4700	948	8 9 9	13	თ •	rO r	6956	ക വ	38		C.T.	
4500 2504 110 16 6 5 10,000 4.1 46 10,100 4.1 46 10,100 10,100 4.1 46 10,100 10,100	D 0	700	1730	103	1.4 7.1	4. п	U R	1169) ·	14.	!		
Secondary 153 16 6 5 10,608 4.8 46 4.0 4.8 4.8 4.8 4.8 4.8 4.8 4.0 4.8 4	1-8 1-9	4500	2506	141	16	0 0	വ	9180	4. 4. 5. 1.	446	1	. T . U	
1900 480 35 13 3 5 6908 3.4 38	6-1	5200	2794	153	16	0	Ŋ	10,608	4.8	46	1	the same	
37,300 752 52 13 3 5 7844 3.9 38 22 2500 288 23 13 3 5 7992 4.0 38 30' - 54" 25 5400 652 46 13 3 5 7992 4.0 38 7900 480 35 13 5 5002 2.1 38 3400 604 480 35 5 5022 2.1 38 90' - 60" 4800 756 52 4292 2.1 38 15' C.T. Br.	. 1-1	4600	480	35	13	က	2	6808	3.4	38	-	1	
2500 288 23 13 3 5 3700 1.8 38 30' - 54" 5400 652 46 13 3 5 7992 4.0 38 7900 Fresent canal as constructed is considered adequate 11,692 5.8 3400 480 35 13 3 5 5032 2.1 38 15' C.T. Br. 2900 604 43 13 3 5 6956 3.5 38 15' C.T. Br. 4700 1320 82 13 5 6956 3.5 38 15' C.T. Br. 3100 128 12 3 5 6956 3.5 38 15' C.T. Br. 18,900 128 12 13 3 5 5022 2.5 38 15' C.T. Br. 18,900 462	1-1 :al-9	5300	752	22	e E	m	n	7844 62,730	0.0°	38	-	1	22,046.00
5400 652 46 13 3 5 7992 4.0 38 7900 Present canal as constructed is considered adequate 11,692 5.8 30' - 60" 3400 480 35 13 3 5 4292 2.5 38 15' C.T. Br. 4800 756 52 13 3 5 4292 3.5 38 15' C.T. Br. 4800 756 82 13 3 5 4292 3.5 38 15' C.T. Br. 4700 1320 82 13 5 4292 3.5 38 15' C.T. Br. 18,900 128 82 13 5 4588 2.3 38 15' C.T. Br. 18,900 462 34 13 5 5032 2.5 38 15' C.T. Br. 1000 462 34 13 <td< td=""><td>1-10</td><td>2500</td><td>288</td><td>23</td><td>13</td><td>8</td><td>5</td><td>3700</td><td>1.8</td><td>38</td><td></td><td>- 54</td><td></td></td<>	1-10	2500	288	23	13	8	5	3700	1.8	38		- 54	
7900 480 35 13 3 5 6032 2.5 38 30' - 60" 2900 604 43 13 3 5 4292 2.1 38 30' - 60" 4900 604 43 13 3 5 4292 2.1 38 15' C.T. Br. 4700 1320 82 13 3 5 6956 3.5 38 15' C.T. Br. 310 128 12 3 5 4588 2.3 38 15' R.C. Br. 18,900 316 Present canal as constructed is considered adequate 27.972 13.9 </td <td>1-10</td> <td>2400</td> <td>652</td> <td>46</td> <td></td> <td>m</td> <td>5</td> <td></td> <td></td> <td>38</td> <td>!</td> <td>-</td> <td></td>	1-10	2400	652	46		m	5			38	!	-	
3400 480 35 13 3 5 5032 2.5 38 30' - 60" 2900 604 43 13 3 5 4292 2.1 38 15' C.T. Br. 4800 756 52 13 3 5 6956 3.5 38 15' C.T. Br. 4700 128 12 13 3 5 6956 3.5 38 15' R.C. Br. 18,900 128 12 13 3 5 5032 2.5 38 15' R.C. Br. 18,900 412 31 3 5 5032 2.5 38 20' - 36" 11 5200 894 60 13 3 5 7696 3.8 15' C.T. Br. 2800 1050 68 13 5 1444 2.1 38 15' C.T. Br.	1-10 tal-10	7900		D 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0				\vdash			 		4203.00
2900 604 43 13 3 5 4292 2.1 38 15° C.T. Br. 4800 756 52 13 3 5 6956 3.5 38 15° C.T. Br. 4700 1280 12 3 5 4588 2.3 38 15° C.T. Br. 18,900 412 12 3 5 5032 2.5 38 17° C.T. Br. 1000 462 34 13 3 5 1480 0.7 38 20° - 36" 5200 894 60 13 3 5 7696 3.8 15° C.T. Br. 2800 1050 68 13 3 5 4144 2.1 38 15° C.T. Br. 12.400 1050 68 13 2 4144 5.1<	4-11	3400	480	35	13	8	5	5032	2.5	38			
4800 756 52 13 3 5 7104 3.5 38 15' C.T. Br. 4700 1320 82 13 5 6956 3.5 38 15' C.T. Br. 3100 128 12 3 5 4586 3.6 15' C.T. Br. 18,900 412 31 13 5 5032 2.5 38 20' - 36" 1000 462 34 13 3 5 7696 3.8 15' C.T. Br. 2800 1050 68 13 3 5 1444 2.1 38 15' C.T. Br. 12,400 1050 68 13 3 5 1444 2.1 38 15' C.T. Br.	1-11	2900	604	43	13	m	D.	4292	2.1	38	1		
18,300	4-11	4800	756	0 02	H 6	ကင	n u	7104	о п	38	-	C.T.	
18,900 412 31 13 3 5 5032 2.5 38 20' - 36" 11 3400 462 34 13 3 5 5032 2.5 38 20' - 36" 11 5200 462 34 13 3 5 1480 0.7 38 20' - 36" 5200 894 60 13 3 5 7696 3.8 15' C.T. Br. 2800 1050 68 13 3 5 4144 2.1 38	1-1-	3100	128	12	13 13	n m	о О	4588	ກ ຕ ກ ໙	38		١. د	
16,900 412 31 13 3 5 5032 2.5 38 20' - 36" 1000 462 34 13 3 5 1480 0.7 38 5200 894 60 13 3 5 7696 3.8 15' C.T. Br. 2800 1050 68 13 3 5 4144 2.1 38 15' C.T. Br. 12,400 18,352 9.1	L-1	(316	Presen		s cons	tructe	considere	adequat			-	6
3400 412 31 13 3 5 5032 2.5 38 20' - 36" 1000 462 34 13 3 5 1480 0.7 38 20' - 36" 5200 894 60 13 3 5 7696 3.8 38 15' C.T. Br. 2800 1050 68 13 3 5 4144 2.1 38 15' C.T. Br. 12,400 9.1 18,352 9.1 15' C.T. Br.	tal-11	18,900						27,972	13.9				11,482.00
5200 894 60 13 3 5 7696 3.8 38 15° C.T. Br. 2800 1050 68 13 3 5 4144 2.1 38 15° C.T. Br. 12,400 12,400 18,352 9.1 <	1-12	3400	412	31	13	ന ന	വവ	5032	2.5	88 88		ıj	
2800 1050 68 13 3 5 4144 2.1 38 12,400 12,400	1-12	5200	894	9	13	n	Ŋ	7696	9°8	38	-	C.T.	
12,400	4-12	2800	1050	68	13	က	IJ	4144	2.1	38	1		
	tal-12	12,400						18,352	9.1				6520.00

ENGINEERING AND DESIGN DATA

ESTIMATED COST Sheet 6 of 6 9520.00 4513.00 652,137.00 4805.00 3047.00 Dollars (13) TOTAL CULVERTS & BRIDGES - NEW Length & Size Br. Br. Br. Br Br. - 24" U.T. R.C. U.T. U.T. (12) U.T. 40. 15' 151 151 151 CULVERTS LOWERING Length & Size $\widehat{\Xi}$ | | | | | | | 1 1 1 1 1 Area 3 · Wassamassaw · Cooper's Store · New Hope · Lebanon REQUIRED RT. OF WAY WIDTH Ft. (10) 38 38 38 38 38 38 38 44 38 38 RT. OF WAY CLEARING Ac. (9) 1.0 3.2 0.7 3.6 2.3 3.2 2.0 1.2 4°. ô 850 EXCAVATION 4144 9324 2368 6808 9176 4292 4588 13,764 6512 1332 7585 1,962,278 2072 4588 6512 Cu. Yds. (8) 28,601 AVERAGE DEP TH Ft. (7) CHANNEL DINENSIONS 10 LO 10 LO 01 01 01 01 01 01 01 01 BO TTOM WIDTH `Ft. (6) თ თ ကက თ თ თ 0000000 TOP Ft. (5) 13 13 13 13 13 13 13 13 DISCHARGE c. f. s. (#) 53 15 13 32 13 41 64 82 82 113 24 30 WATERSHED 772 1310 1898 308 400 80 148 420 148 584 972 Ac. (3) 3500 6300 1600 4600 6200 2900 3300 3100 9300 1400 4400 900 4100 3100 4400 18,300 LEN GTH 794,000 Ft. (2) Total-13 Total-14 Total-15 Total-16 M-14 M-14 M-13 M-13 M-15 M-15 L-1 M-16 M-16 M-16 M-16 CANAL Area 3 L-1 L-2 . : : Grand Total ENGINEERING AND DESIGN DATA Area 4 - Sand Ridge - Pringletown

Sheet Lof 3	TOTAL	ESTIMATED COST	Dollars (13)												-					31,094,00			3578.00				0	TO, 2.17, 00					9272.00								13,848.00				((7489.00
		CULVERTS & BRIDGES - NEW	Length & Size (12)		-	1	-	1	1	1	-	30' - 60"				20' C.T. Br.		30' - 48"	1		www.cod	1		30' - 48"	J. J	50' - 54"	1		Ì	60' - 54"		i I	1	60' - 24"	301 - 36"	15' U.T. Br.	FOI C.T. Br.	50 54		1			1	ì	30' - 36"	
		CULVERTS	Length & Size (11)		1	-	1	1		1	1	!	1	I	1	1	1	1 1			1	1			1	1			1	!	-	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	ı			:	!	1		1			50' - 72"	1		
	REQUIRED	RT. OF WAY WIDTH	Ft. (10)	38	41	49	49	57	38	38	38	38	38	41	46	49	8 0	D 00)		38	38		38	38	38	99		38	800	D 00	D 00		38		38	D (n a) (C)	38		38	38	38	38	
		WAY	Ac. (9)	2.8	1.5	4.8	ထ <u>့</u> က	4.3	1.7	1.8	2.1	0.7	2.8	1.4	0.8	o (o √	ກ c	0 4 0 0	2	41.7	2.9	3.0	5.9	4.3	တ္တ	0 :	, v	13.1	3.8	4.1	φ C	1 t	12.4	2.5	,	ش ر در ر	π · ·	9.7	r & .	8.0	17.6	2.5	3.0	2.4	1, 0 2	0
		EXCAVATION	Cu. Yds. (8)	5624	3173	10,878	9768	10,286	3404	3552	4144	1480	5624	2839	1836	8880	2004	8436)	90,284	5920	6068	11,988	8732	6660	6068	20032	20,492	7696	8288	37.00	3404	25,160	5032		6512	3002	6808	7400	5180	35,224	5032	6068	4884	3700	19,084
0 10 10	ONO	AVERAGE DEP TH	Ft. (7)	5	20	Ŋ	Ŋ	ıΩ	Ŋ	Ŋ	72	īΟ	ſΩ	N	ι Ω	υı	Ωи	ט גט)		D.	Ŋ		വ	Ŋ	ro ı	n		2	ro n	Эи	ט ני)	5		Ωı	Ωи	ט ני) IC	ι.		22	Ŋ	ro I	ഗ	
4	DIMEN	¥ -	Ft. (6)	8	4	7	£-	10	n	n	ന	m	က	4	© 1	L (n 0	ാ ന)		8	m		က	m	m (n		က	m (n 0	ים מי)	က		m (ים פי	n m	o m	က		က	က	က	ന	
2 2 4 11 2	CHARREL	WIDTH	Ft. (5)	13	14	17	17	20	13	13	13	13	13	14	16	17	J L	n ⊢)		13	13		13	13	13	უ ⊣		13	13	ე - ე ლ	J F)	13	,	n (o + -	ე ლ ე ლ	ο e.	13		13	13	₩ 13	13	
		DISCHARGE	c. f. s. (4)	37	70	111	122	221	27	23	15	31	46	70	46	917	Ω C	n m)		21	38		17	38	O (,0		26	41	4. r. 0. c	5	1	16	ł	224	, c	20) m	18		13	43	54	o o	
		WATERSHED	Ac. (3)	536	1088	1872	2112	4296	356	288	172	416	716	1088	1584	2004	O C C C	444	1		264	524		200	516	876	TOZOT		340	200	00/	124	1 2 1	184	1	322	000	1240	288	220		152	612	796	85	
		LENGTH	Ft. (2)	3800	1900	4900	4400	3700	2300	2400	2800	1000	3800	1700	006	4000	1800	5700)	50.300	4000	4100	8100	2900	4500	4100	2400	17,800	5200	5600	0000	2300	17,000	3400	(4400	00 1 2	0000	5000	3500	23,800	3400	4100	3300	2500	T 0.000
		CANAL	No.	M-1	M-1	M-1	M-1	M-1	L-1	L-2	L-3	L-3	L-3	Ľ-3	n - 1	L-3	L - 4	בו ווו		Total-1	M-2	M-2	Total-2	M-3	$\mathbb{Z}_{-\infty}$	e 0 − ∑	η	TOURTHR	M-4	M-4	M-4	1 1	Total-4	M-5	l.	Ω L I Σ :	2 E	Z Z	T-1	1-2	Total-5	M-6	M-6	M-6	L-1	iorai-o

ENGINEERING AND DESIGN DATA Area 4 - Sand Ridge - Pringletown

Sheet 2 of 3	TOTAL	COST	0011ars (13)									18.552.00											18,222.00			4074.00								14,462.00							
	, , , , , , , , , , , , , , , , , , ,	BRIDGES - NEW	Length & Size (12)	11 151	.	4		1	i	ŀ	30' - 48"	ero ma		-	15 t II T Bx		}		-	1		30' - 36"		1	1	30' - 60"			15' C.T. Br.		15' U.T. Br.		40' - 18"		40' - 48"		· ·		T .	30' - 30"	1
		LOWERING	Length & Size (11)			-	1	-	-	-	-	i I		and the			1	1	1	1				an	1			1	1			-	-	!							
a l	REQUIRED	WIDTH	Ft. (10)	38	94	46	49	57	38	38	80 00	0	38	88	38	46	44	38	38	38	0 0	D 00 7 07			38		38	38	14	, da	38	38	80 0 80 6	0	38	m m	0 00	40	62	38	0
Fringletown	3	EARING	(9)	22.53	1.2	2.0	3.5	3.0	4.1	2.1	o r	28.0	4.9	1.0	н к т	1.7	1.7	2.8	8,8	, is	D C	. i.	28.2	adequate	6.8	8.9	3.6	4.0	7.00) c	0.4	0.5	1. 1.	20.7	0.4	D 0	1 · ·	. w	7.5	e, t	
Sand Kidge - F		EACAVALIUR	Cu. Yds. (8)	5032	2652	4488	7770	7228	8288	4292	5772	59,138	9916	2072	3700	3672	3515	5624	4440	5032	3073	3552	58,311	s considered	5772	5772	7252	8140	5678	1184	7992	5772	2220	41,938	8140	07.7.7	2000	12,852	18,270	2664	2 T 22
4 - San		DEPTH	Ft. (7)	ממ	10	Ŋ	Ŋ	Ŋ	Ŋ	Ŋ	א טו)	Ŋ	ın ı	Ω IΩ	Ω (Ŋ	Ŋ	Ŋ	IO II	יו כ	വറ		tructed i	Ω		5	Ŋ	ıo u	ט נמ	Ω	ıs ı	n n)	וטו	טוט) וכ	വ	D	n n	o
Area	NEL DIMEN	WIDTH	۲t. (6)	(m m	9	0	7	10	ന	ന	m с	0	e	es (n «	9	5	က	ന	ന	o c	n m		as cons	8		8	က	4 0	റ ന) m	ന	n c	o	(n)	n (n	o er.	0 0	12	m 0	2
	CHANNE	WIDTH) (5)	13	18	16	17	20	13	13	13	2	13	13	18	16	15	13	13	13	J L	n m		nt cana]			13	13	14	J F	13	13	n e) H	13	ກ ⊏	- F	19	22	13	o ⊢
	4 3 6 6	O SCHARGE	с. т. s. (ц)	888	88	95	115	145	33	17	0 00 0	r '	33	88 1	4 Q	112	115	14	17	ლ (0 0) H		Presen	34		34	54	71	ο α Η α	08	12	6	-	28	J 10) m	105	169	10	C.T
	6 6 6 1	WALERONEU	Ac. (3)	368	1420	1556	1964	2580	440	204	240	200	456	524	784	1916	1996	168	204	4 38 8	900	112		228	468		468	800	1116	1490	240	128	116	GTT.	368	400 008 008	0 0	1760	3088	108	184
	2	5 . E !	Ft. (2)	3400	1300	2200	3500	2600	5600	5900	3900	34.600	6700	1400	2500	1800	1900	3800	3000	3400	1400	2400	36,200		3900	3900	4900	5500	3400	800	5400	3900	1500	27,900	5500	0086	0000	6300	2800	1900	0061
		CARAL	(1)	M-7	M-7	M-7	M-7	M-7	L-1	L-2	0 °	Total-7	M-8	M-8	00 00 1 2	0 00	M-8	L-1	L-2	L-2	n	L-1	Total-8	6-M	6-M	Total-9	M-10	M-10	M-10	M-10	L-1	L-2	г Г Г	Total-10	M-11	M-11	Z	M-11	M-11	L-1	1 – 1

ENGINEERING AND DESIGN DATA Area 4 - Sand Ridge - Pringletown

Sheet 3 of 3

TOTAL	ESTIMATED	Dollars (13)	26,261.00	3365.00	3104.00	163,528.00
	CULVERTS & BRIDGES - NEW	Length & Size (12)	 15' C.T. Br.	40' - 36"	30' - 42"	
	CULVERTS	Length & Size (11)		1 1	50' - 36"	
REOUIRED	>-	Ft. (10)	38 88 88	38	38	
	RT. OF WAY	Ac. (9)	2.6 3.5 3.5 1.1	2.3	3.7	222.77
	EXCAVATION	Cu. Yds. (8)	5328 6956 7104 73,154	4588 4736 9324	7548 7548	464,017
SIONS	AVERAGE	(7) Ft.	מממ	ממ	ro.	
CHANNEL DIMENSIONS	BO T T O M	Ft. (6)	0°0°0°	നന	m	
CHANN	TOP	Ft. (5)	13 13	13	13	
	DISCHARGE	c. f. s. (4)	3 2 3 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	33 S	50	
	WATERSHED	Ac. (3)	240 476 952	316	720	
	LEN GTH	Ft. (2)	3600 4700 4800 40,500	3100 3200 6300	5100	284,900
	CANAL	No.	L-2 L-3 L-3 Total-11	M-12 M-12 Total-12	M-13 Total-13	Area 4 Grand Total

ENGINEERING AND DESIGN DATA

Area 5 - Cross

Sheet 1 of 3	TOTAL	ESTIMATED COST	Dollers (13)			-	1	13,295.00						13,794.00		0000	000			3246.00					8034.00				`			13,144.00		-			6521,00					
She		RTS &				5' R.C. Br.		13		15' R.C. Br.	1	15' R.C. Br.	! !	13	5' R.C. Br.	!		107		-	15' U.T. Br.	U.I.	U.T.	15' R.C. Br.	-		30' - 48" 15' C.T. Br.		15' C.T. Br.	i.	30 30.:	. 13	1	1	30' - 60"	1		30' - 24"	I	1	30' - 42"	
		TS NG BF	9 2			1	!							•	-	-		1	l			1			-									-	l I	1	!					
	REQUIRED	>	Ft. (10)	44	52	55	38		38	38	41	46 38	380		38	38		00 a) @		38	38	38	38	38		00 00 00 00))))	38	940	D 00	0	38	38	38	m a	0	38	38		m c	0 0
S		¥ΑΥ NG	Ac. (9)	4.9	0.	1.9	. s. r.	17.3	3.5	2.5	0. 0. 0. 0	0. w	o eo eo	17.6	5.3	v 00		0.1) o	4.4	3.9	1.0	1.5	0.3	U 00 €.		m 0.	2	1.5	8 4	r. c	15.2	2.2	0.2	0.0	on 12	0.8	0.7	2.2		2.0) t
sa 5 - Cross		×	Cu. Yds. (8)	10,360	16,058	4403	7104	37,925	6956	4440	5845	7140	6660	36,369	10,656	4588	10.01	2072	3848	9028	7844	2072	3108	592	3256	2	10,656	1	2960	4080	2000	30,868	4440	444	3996	1939	15,984	1480	4440		1332	7 1
Area	SIONS	AVERAGE DEP TH	Ft.	22	5	22	2		2	ιΩ	ι Ω	n n	ט נט		2	ro		n n	ט נט		2	5	2	Ŋ	ಬ	1	n n)	υ	ro r	Оп	י	5	2	ហេរ	o r	2	2	2	1	υи) l
	EL DIMENSIONS	2	Ft. (6)	υ	0	o.	က		e	n	4 (υ r	ာက	-	m	ന		თ ო) m		m	က	က	က	ന		ന ന)	ო	φ (n 0	ว	e	က	m (ന ന	n	m	m		ന ര	o (
	CHANNE	TOP WIDTH	Ft. (5)	15	19	19	13		13	13	14	16	13		13	13		13	H H		13	13	13	13	13		T T T)	13	16	LG C	21	13	13	13	13	2	13	13	,	T T) t
		DISCHARGE	c. f. s. (4)	58	102	109	34		27	63	76	თ ი თ ი	222		40	49		0 C	15		33	50	56	58	11		4 8 4 4	1	49	74	/,	, 1,	13	1.7	600	т Э	2	5	12	:	9 10	- C
		WATERSHED	Ac. (3)	860	1700	1840	460		360	958	1182	1650	276		556	969		40	170		440	720	840	864	124		460)	708	1150	200	O H	140	204	380	540	7,	48	132		188	0 0
		-	Ft. (2)	5600	6200	1700	4800	18,300	4700	3000	3500	3500	4500	22,800	7200	3100	TO, 300	1400	\$ TOO	6100	5300	1400	2100	400	2200	11.	7200		2000	2000	3000	20,100	3000	300	2700	3800	10,800	1000	3000		900	000
		CANAL	.()	M-1	M-1	M-1		Total-1	M-2	M-2	M-23	M-2	L-1 L-2	Total-2	M-3	M-3	TOTALLO	M-M	I. = 1	Total-4	M − 5	M-5	M-5	M-5	Total -5	0 1 4 6 5 0 1	9 W W X)	M-6	M-6	1-1	L-1 Total-6	M-7	M-7	M-7	M-7	L-1 Total-7	M-8	M_8		∞ c	0 0

ENGINEERING AND DESIGN DATA Area 5 - Cross

Sheet 2 of 3	TOTAL	ESTIMATED COST	Dollars (13)		6928.00							00.9888					2280.00			3994.00												,	,		21,643.00							
		S -	Length & Size (12)	30' - 18"		30' - 30"		15' U.T. Br.	40' - 30"	LO C.T. Br.				30 36.				30' - 48"				15' C.T. Br.	1	R.C.	15' C.T. Br. 15' U.T. Br.		6	Lo' C.T. Br.			30' - 42"	1	1	7505	1			40' - 48"		15' C.T. Br.	1	30' - 42" 60' - 30"
		CULVERTS	Length & Size (11)			-	-	-		1 1		1			. !	20' - 18"					1	1	1	1		1	1				1	-	-	1				-	-		1	
	REQUIRED	RT. OF WAY	Ft. (10)	38	88	38	38	38	C	D 00	0 8	38		20 a	0 8	38		38	38		38	38	41	46	46	46	49	20	0 00	9 8	38	38	C	200		38	38	38	41	44	46	8 8
9		RT. OF WAY	Ac. (9)	0.0	യ് യ ⊷് യ	2.6	0.7	1.0	0	0 4	1 00	11.0	1 0	0 0	. H	0.5	3.0	3.5	2.1	5.6	2.6	0.6	0.8	0.4	1.8	1.4	\ \ \ \ \ \	o «	2 0	≥ 5	1.8		ade	ر بوروم م+وروم م+وروم		3.7	2.0	0.4	4.0	4.0	φ. α α	O. m
ea o - Cross		EXCAVATION	Cu. Yds. (8)	1628	3700	5180	1480	1924	0	1020 4884	5772	2072	7	1184 888	2960	1036	6068	6956	4292	11,248	5180	7844	1670	816	4080	6120	2664	1330 0017	4440	3663	2664		is considered	0980	. 4	7548	740	1332	8350	925	5712	8008
Area	SIONS	AVERAGE DEP TH	Ft. (7)	22.	ω	5	ro	Ω	и	о го	υ Ω	Ω	L	o и	ω Ω	22		2	Ω		ro	ιO	υ	ro	ıΩ	ις	Юп	οи	ט וכ	2	ಬ	ت	constructed	0		5	22	2	22	10 1	ı کا	Ω
		BO TTO M WIDTH	Ft. (6)	en (m	ന	ന	C	n m	ന	თ		ກ ຕ) ന	က		m	ന		ന	က	4	9	ø	0	L (OT 0"	o 00	ന	ო	n	1 ass	ט מ	d d	m	m	ന	4	το (9 (ກ
	CHANNE	TOP WIDTH	Ft. (5)	13	13	13	13	13	7	n ←	13	13	7	n ∈	13	13		13	13		13	13	14	16	16	16	17) C	- F	13	13	13	int cana	_ L3 _		13	13	13	14	15	16	T3
		DISCHARGE	c. f. s. (4)	€ 1	വ	13	27	38	2	, rc	14	က		σο σο	15	က		20	27		20	52	09	72	94	81	, 0 0 0	T 30	γ σ:) O	1.7	7	Present	CC	0 14	18	31	43	61	71	78	C C
		WATERSHED	Ac. (3)	18	44	154	358	534	0	0.70	164	28		4 C	182	24		248	358		252	756	892	1118	1194	1302	1552	2800	000	88	196	68	220	200	0	220	415	605	925	1105	1245	180
		LENGTH	Ft. (2)	1100	2500	3500	1000	1300	7	3300	3900	15.500		000	2000	700	4100	4700	2900	7600	3500	5300	1000	400	2000	3000	1200	2000	0000	3300	2400	2200	()	4000	35,300	5100	200	006	2000	500	2800	4100
		CANAL	No.	L-1	L-2 Total-8	W-9	M-9	M-9	;	D 0	L-1	L-2 Total-9		M-10	M-10	L-1	Total-10	M-11	M-11	Total-11	M-12	M-12	M-12	M-12	M-12	M-12	M-12	M-LZ	- C	∏-3	L-3	L-4	L-5	1 P	Total-12	M-13	M-13	M-13	M-13	M-13	M-13	

ENGINEERING AND DESIGN DATA Area 5 - Cross

Sheet 3 of 3	TOTAL	ESTIMATED COST	Dollars (13)			13,669.00				5160.00						13,228,00									7119.00									_			34,757.00		181,713.00		
		S & WEW	Length & Size (12)	1	30' - 36"		40' - 48"	15' C.T. Br.			ĺ	40' - 54"	1 6	60' - 30"	1	40' - 24"	The contract	}			15' R.C. Br.	-	1		i i				IS' C.T. Br.	and days	30' - 60"	-	-		E	ro. U.T. Br.					
		CULVERTS	Length & Size (ii)		1		1	!	-			!	1		!			-	-	!	!		-	1			-	!	!	1	1	!	!	-	!	1					
	REQUIRED	RT. OF WAY WIDTH	Ft. (10)		38		38	38	38		38	41	44	88	38							38		O C	0	38	38	38	ď	73		38	38	62	N (0	300	- •				
		W A Y	Ac. (9)	ed adequate		16.9	2.5	1.0	4. 8	20.0	57.3	0 C		3 0	1 .2	16.4	ed adequate	ed adequate	ed adequate		(0			ಹ	, o	4.3	6.1	3.5	er,	0.8	w		5.4	4.0	w z	4 w	47.3		229.2		
a 5 - Cross		EXCAVATION	Cu. Yds. (8)	is considered	5180	35,855	5032	2072	4884	11,988	10,656	3841	2000	5180	3848	35,499	is considered	1.5	is considered	Ţ			. I.S	1s considered	18,796	8584	12,284	6364	0999	20,228		11,100	10,804	945	7875	9504 77.17	100,943		478,126		
Area	SIONS	AVERAGE DEP TH	Ft. (7)	constructed	D		ъ	Ŋ	വ		ιΩ	IO I	υп	o ro	Ŋ		constructed	constructed	constructed	constructed	constructed	വ	constructed	constructed)	5	Ŋ	ıQ	ıc	ω	constructed	Ω	വ	ıΩ ı	Ωи	n n)				
	CHANNEL DIMENSIONS	W1DTH	, Ft. (6)	as	က		က	က	က		က	4, ι	ດ ແ) m	က		8	28	s S	ಜ	s S	က	d d	თ ი)	9	ო	က	cr.	16	98	n	ო	12	77 (n <	Н				
	CHANN	WIDTH	Ft. (5)	Present canal	13		13	13	13		13	14	T မ	13	13		Present car	Present car	Present canal	Present car	Present car	13	Present canal	sent car 12) H	13	13	13	φ.	26	Present canal	13	13	22	2 2 2	٦ ل	r H				
		DISCHARGE	c. f. s. (+)	Pre	14		31	37	47		36	20	00	15	13		Pre	Pre	Pre	Pre		o i	14 th	Pre	0	0	17	28	en en	257	Pre	10	17	110	E C C	υ ς υ σ ζ	r r				
		WATERSHED	Ac. (3)	170	160		408	508	664		200	832	896	180	148		732	2392	2564	4104	4468	580	292	2002	000	584	1236	2200	089%	5148	676	672	1232	1856	1928	4 0 0	0				
		±	Ft. (2)		3500	22,400	3400	1400	3300	8100	7200	2300	1400	3500	2600	21,600						8900		C	12,700	5800	8300	4300	4500	5200		7500	7300	300	22200	2800	56,000		295,000		
		CANAL	No.	1-2	L-3	Total-13	M-14	M-14	M-14	Total-14	M-15	M-15	M-15	I I I	L-2	Total-15	M-16	M-16	M-16	M-16	M -16	Г-1 1	N (L - 32	Total-16	M-17	M-17	M-17	M-17	M-17	L-1	1-2	1-2	1-2	N (n 0	Total-17	Area 5	Total		

ENGINEERING AND DESIGN DATA Area 6 - Eadytown - Pineville - St. Stephen

TOTAL	ESTIMATED COST	Dollars (13)				
 | | | | |
 | | | | | |
 | | | | |
 | | | | |
 | | | | | 70 07 NO
 | | | | | | | |
 | | | |
 | |
|----------|-----------------------------|--|---|---|---|---
---|---|---|---
--|--|--|--|--------------------
--	--	--	--
--	---	--	--
--	--	---	--
---	--	---	--
--	--	--	--
--	--	--	--
	F .	2 (2	
 | C. H | R. C. | - | - | 40' - 54"
 | 15' C.T. Br. | ł | I | I | ı |
 | | 40' - 30" | - | 15' U.T. Br. | 1
 | | l | 1 | 1 | 1
 | ì | 20' - 48" | 1 | | 1
 | | 1 | | - | 15' R.C. Br. | - | U.T. | C.T.
 | | | 1 | 1
 | 1 |
| | CULVERTS | Length & Size
(11) | ommended | | ! | ! | !
 | | ! | - | 1 |
 | 1 1 | 1 | - | | 1 1 |
 | ! | 120' - 36" | - | ! | -
 | ł | I | | |
 | - | 1 | 1 | ! | !
 | | | | - | 1 | 1 | 1 | !
 | - | 1 | - | 1
 | |
| REQUIRED | RT. OF WAY
WIDTH | Ft.
(10) | are | 38 | 38 | 44 | 49
 | 68 | 55 | 46 | 62 | 38
 | 38 | 38 | 88 | m (| 0 8 | 0 8
 | 38 | 41 | 38 | 44 | 44
 | 38 | m (| m a | S & | 0 8
 | | | | 38 |
 | | | | | | | 44 | 46
 | 38 | 38 | 88 6 | 00 00
00 00
 |) |
| | RT. OF WAY | Ac.
(9) | | 1.6 | 1.0 | 2.7 | 1 .6
 | 11.7 | 1.8 | 5.2 | 2 .7 | 1.9
 | 1.6 | T. (| n (| ω -
ν (|
- r. | 4.0
 | 0.5 | 8.8 | 8.8 | 3.5 | ල
ග
 | 1.6 | 0 0 | n 0 | s C |) Q
 | | | | |
 |) I | | | | | | 4.4 | တ္
 | 1.8 | o. 0 | 7.0 | 03 ←
03 0
 |)
+ |
| | EXCAVATION | | Moultrie | 3256 | 2072 | 5735 | 3552
 | 28,864 | 4144 | 11,628 | 6615 | 3848
 | 3256 | 2220 | 2920 | 1024 | - CA02 | 888
 | 1036 | 6847 | 5624 | 7400 | 8325
 | 3256 | 1164 | 2002 | 1038 | 6364
 | | 13 | | |
 | 1 I | | 1.5 | | | | 9435 | 2967
 | 3552 | 5772 | 740 | 98840
 | #
0
0
2
2 |
| SIONS | AVERAGE
DEP TH | Ft. (7) | in | 5 | 23 | 22 | 22
 | 5 | 22 | 5 | 20 | 20
 | ro i | 10 I | O I | Оп |) IC | . ro
 | 22 | 2 | 22 | Ω | ıo ı
 | n u | о и | ດທ |) rc | on (د
 | onstructe | onstructe | onstructe | ص.
د | onstructe
 | + 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 | onstructe | onstructe | onstructe | onstructe | onstructe | Ω | N
 | N | រប រ | O F | ນເ
 |) |
| E E | 80 TTOM
WIDTH | Ft.
(6) | on of wat | ,
CO | თ | N | 7
 | 14 | o | 9 | 12 | ന
 | თ (| m (| m (| n (1 | o m | n m
 | m | 4 | თ | N | n (
 | ကပ | n 0 | n a | o m | n m
 | SS | 8 | s
S | | ರ
ರ
 | 1 | 0
0
0 | м
8 | 8 | 8 | ಚಿ | Ŋ | ဖ
 | m | ကေ | n c | ന ന
 |) |
| CHAN | | Ft.
(5) | elevati | 13 | 13 | 15 | 17
 | 24 | 19 | 18 | 22 | 13
 | 13 | | Γ . |
ກ ເ | - H | 13
 | 13 | 14 | 13 | 15 | 13
 | T 7 | T F | n ⊢ | - H | η
Η
 | O | | | |
 | - 1 |) (| U | O | 0 | O | 15 | 16
 | 13 | 13 | Τ - F | 1 T
 |)
H |
| | DISCHARGE | C. f. 8.
(4) | to normal | 10 | 12 | 53 | 69
 | 121 | 124 | 138 | 233 | 24
 | 37 | 8 6 | 0 % | Λ
I π | 10 | (((((((((((((((((((
 | 10 | 45 | 62 | 80 | 112
 | ∞ ¬ | 1 , n | റെ ത | > 4 | 19
 | | Д | | | 14
 | | , D. | Д | Д | Д | | 106 | 153
 | 12 | 88 | N C | n a
 |) |
| | WATERSHED | Ac.
(3) | Due | 100 | 132 | 774 | 1060
 | 2094 | 2142 | 2458 | 4584 | 304
 | 504 | 530 | 042 | у
4 С | 110 | 56
 | 104 | 644 | 926 | 1286 | 1882
 | 76 | 0.4 | φ. α
α | 0.7 | 224
 | 99 | 318 | 376 | 496 | 140
 | c
our | 2000 | 1242 | 1360 | 1520 | 1594 | 1790 | 2790
 | 136 | 296 | 16
08.4 | 25 d
 | 1) |
| | LEN GTH | Ft.
(2) | | 2200 | 1400 | 3100 | 1600
 | 8200 | 1600 | 5700 | 2100 | 2600
 | 2200 | 1500 | 4000 | 1300 | 2000 | 009
 | 700 | 4100 | 3800 | 4000 | 4500
 | 0000 | 000 | 3000 | 300 | 4300
 | | | | 3400 | 26.300
 | | | | | | | 5100 | 3900
 | 2400 | 3900 | 0000 | 1800
 |) |
| | CANAL | No. | M-1 | M-2 | M-2 | M-2 | M-2
 | M-2 | M-2 | M-2 | M-2 | L-1
 | L-1 | L-1 | L-12 | 1 L S | 1,-4 | L-5
 | L-5 | L-5 | L-5 | L-5 | L-5
 | L-6 | 7-1 | L-7 | α | . 8-1
 | 1-9 | L-9 | L-9 | L-9 | L-10
 | 2 00 | 0 C | M-3 | M-3 | M-3 | M-3 | M-3 | ε - Σ
Σ
 | L-1 | 2-13
1-13 | L-13 | L-3
 | 4 |
| | CHANNEL DIMENSIONS REQUIRED | LENGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY CULVERTS & ES WIDTH WIDTH WIDTH DEPTH | LENGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY CULVERTS & CULVERTS | LENGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY CULVERTS & CULVERTS | LENGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY CLLVERTS & CULVERTS | LENGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY RT. OF WAY RT. OF WAY CULVERTS & CULVERTS | LENGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY CULVERTS & | LENGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY CULVERTS & | LENGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY RT. OF WAY CULVERTS & CULVERTS | LENGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY CULVERTS & | LEWGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY | LENGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY | LENGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY | LENGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION CLEARING REQUIRED LOWERTS CLUVERTS CLUVERT | CHANNEL DINERSIONS | LEWGTH WATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY | CLENGTH WATERSHED DISCHARGE TOP GOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WA | LENGTH MATERSHED DISCHARGE TOP BOTTOM AVERAGE EXCAVATION RT. OF WAY RT. OF WAY RT. OF WAY | LENGTH WATERSHED DISCHARGE TOP MATERSHED MATERSHED DISCHARGE TOP MATERSHED MATERSHED DISCHARGE TOP MATERSHED MATERSHED MATERSHED DISCHARGE TOP MATERSHED MATERSHED MATERSHED DISCHARGE TOP MATERSHED MAT | CHANNEL DINERSHED CHANNEL DINERSIONS CHANNEL DINERSINGS CHONNEL DINERSHED CHANNEL DINERSHED CHANNEL DINERSHED CHANNEL DINERSHED CHANNEL DINERSHED CHANNEL DINERSHED CHANNEL DINERSHED C.f. 3. C.f. 3. Ft. Ft. Ft. Cu. Yda. Ac. C.f. 3. Ft. Ft. Ft. Cu. Yda. Ac. C.f. 3. (1) (1) (1) (1) (1) (12) (12) (13) (13) (14) (14) (15) (| Canada Wateshed Mateshed Mateshed | Canadian Watershed Watershed Watershed Watershed Work Watershed Watershed | CLAMAR WATERSHED DISCHARGE CTAMARS CLAMARS C | CLANTRAINED LONGRANGE LO | CLEMENTH WATERSHED DISCHARGE TOTAMH AVERAGE EXCAVATION RT. OF WAY ROUTH ROUTH | Feb. 17 WATERSHED DISCHARGE CANANTIL DIMERSIONS CLEARING WIDTH WIDTH DEFENSE EXCAVATION CLEARING WIDTH WIDTH DEFENSE EXCAVATION CLEARING WIDTH LONG DISCHARGE CLEARING WIDTH WIDTH DEFENSE CLEARING WIDTH LONG DISCHARGE CLEARING WIDTH LONG DISCHARGE CLEARING WIDTH LONG DISCHARGE CLEARING WIDTH DEFENSE CLEARING WIDTH LONG DISCHARGE CLEARING DISCHARGE D | CHANKE DISCHARGE TOP DOTTON AVERAGE EXCAVATION CLEARING NIDTH CLUVERING LONGRING CLUVERING CLUVERING LONGRING CLUVERING CL | Ft. Att. A | Canada C | Fig. Heat Heat | Column C | Ft. Ac. C. f. f. a. C. f. a. C. f. f. a. C. | Fig. 19 SCHANNEE DINERS DINES SCHANNEE DINERS DINES SCHANNEE DINES DINES SCHANNEE DINES DI | Fig. 4. A. A. C. C. A. A. C. A. A | Feb. Ac. C. f. s. F. f. F. f | Fig. 1 | Fig. Ac. C. C. C. C. C. C. C. | Feb. MATERISE DISCHARGE TOP MOTTON M | Feb. WATERSHED DISCHARGE TOP MATERIAL MATERSHED MATE | Character Colorest Colorest | Characteristics Colored Control of Control of Colored Col | Column C | Column C | Characteristic Control Control | Feb. 18 WATERSIED DISCURDANG TOTAL MATERSIED TOTAL WATERSIED TOTAL WATERSIES TOTAL WATERSIES TOTAL WATER | Column C |

ENGINEERING AND DESIGN DATA Area 6 - Eadytown - Pineville - St. Stephen

Sheet 2 of 5	TOTAL	ESTIMATED COST	Dollars (13)					17.941.00						8942.00				6241.00										17,390.00												
		CULVERTS & BRIDGES - NEW	Length & Size (12)	20' - 24"	20, - 30"	1	-		40' - 48"	1	1	ŀ	40 24			15' U.T. Br.	-		- 36	15' U.T. Br.	·Jg ·I·n .cT	15' U.T. Br.		20, - 36"	1	15 11 T Br	1		30' - 24"	15' U.T. Br. 15' U.T. Br.	-		-	1 8	30' C.T. Br.		C.T.	30' R.C. Br.	-	
		2 ×	Length & Size (11)	1	-		1	1	1	1	-	,			40' - 36"		-		1	1		1	1	!			-	1			1		-	!	!		-		-	
Stephen	REQUIRED	RT. OF WAY WIDTH	Ft. (10)	38		4)		D D	38	38	38	38	m cc	9 9 9	38	38	38		38	38	38	44			38	38	38	38	38	38	38	46	46	64	U I	N 00	73	78	84	о Ф
St.		4 A Y	AC. (9)	1.1				23.8	e .e	1.2	0.2	4.0	٠ - C	0. T 81	1.5	3.4	4.2	9.1	8.3	o. o.	+ +	2.4			ლ ₁	-1 4 -23 00	2.4	1.0	0.9	4.8	7. 0	1.5	0.4	ø,	4. T.	ન જ અં વ	. w	1.8	0.5	Φ.
Area 0 - Eadytown - Fineville -	~~	EXCAVATION	(8)	2220		d is considered	d is conside	46,926	6660	2368	444	9028	25.00	2664 25.012	2960	6808	8436	18,204	4588	11,100	2220	5180			6660	2308	4884	2072	1776	9768	1332	3264	816	1776	6388	7040	8169	4586	24,539	2000
Eadytow	DIMENSIONS	AVERAGE DEP TH	Ft. (7)	ಬ	īO	constructed	constructed	0	2	5	ı N	ro r	O 10	o ro	5	ιΩ	ιC		ις.	N	ιΩ	70	constructed	constructed	10 1	o ro	ι.O.	ιΩ	22	Ŋ	2	5	Ω	10 I	Ωı	ט זכ	. rc	Ω	ro.	ഗ
rea 0 -	ایر	WI DTH	Ft. (6)	n		canal as c		າ	m	n	en (m (n m	o m	m	က	ന		0	m 	ო	10	8	canal as c	m (m m) m	m	ю	ო	n	9	0	2	x (10 F	16	18	20	22
V	CHANN	WIDTH	(5)	13			Present c	n H	13	13	13	13	J L	T 13 C	13	13	13		13	13	13	15			13	H F	13	13	13	13	13	16	16	17	00 0	2.4	28	28	30	cg
		DISCHARGE	C. f. s. (4)	4		<u> </u>	T.) V	27	28	38	91	10 cm	J D	11	40	64		15	38	69	118	PI	PI	ee 2	. 0	200	11	2	20	51	95	108	117	135	210	227	273	286	308
		WATERSHED	Ac. (3)	36	94	418	686	042	350	370	524	020	140	25	120	556	978		172	528	1060	2044	36	194	450	718	878	124	54	720	738	1564	1792	2020	2398	2436	4406	5552	5902	6366
		L EN GTH	Ft. (2)	1500	2000			30,300	4500	1600	300	6100	0000	1800	2000	4600	2700	12,300	3100	7500	1500	2800			4500	1500	3300	1400	1200	9800	006	1600	400	800	3800	1200	2100	1100	5300	400
		CANAL	No.	L-5	P-7	L-7	L-7	L-8 Total-3	M-4	M-4	M-4	M-4	1-1-	L-2 Total-4	- 1	M-5		Total-5	M-8	M-6	Σ	M-6	L-1	L-1	1-1	2 - 1 - 2	2 c c c c c c c c c c c c c c c c c c c	L-3 Total-6	M-7	M-7	M-7	M-7	M-7	M-7	M-7	M - 7	M-7	M-7	M-7	M-7

ENGINEERING AND DESIGN DATA

Sheet 3 of 5 TO TAL ESTIMATED 87,056.00 Dollars CO ST (13) Br. BRIDGES - NEW Length & Size C.T. Br. Br U.T. Br. 15' U.T. Br. U.T. Br. U.T. Br. - 42" - 42" CULVERTS & - 48" - 48" 36" - 42" - 30" - 36" - 24" - 36" - 30" - 24" - 30" - 18" - 42" C.T. U.T. (12) 20. 401 20, 30 201 20. 201 201 201 201 301 401 201 404 15' 1 15' 151 15, LOWERING Length & Size CULVERTS 36" - 18" 40' 20 REQUIRED RT. OF WAY WIDTH Ft. Area 6 - Eadytown - Pineville - St. Stephen CLEARING 10.0 0.4 8.9 0.0 2.1 1.2 (6) Ac. EXCAVATION Cu. Yds. 5032 3404 5772 5772 6364 1924 6808 5624 1036 2505 1036 1036 1332 2812 2368 6216 6660 3848 2812 4144 1480 1336 1110 1020 1480 3848 2072 4144 26,978 7992 740 5920 3841 740 1184 12,876 1184 242,987 (8) BOTTOM AVERAGE . ب DEP TH CHANNEL DIMENSIONS (7) WIDTH (9) 26 WIDTH 36 113 113 113 113 113 TO P (2) DISCHARGE c. f.s. (4) 46 87 22 35 35 37 19 18 14 44 WATERSHED 500 1414 278 482 1460 208 312 608 872 964 1180 1382 8188 506 224 154 618 722 200 176 848 978 82 38 82 128 748 1470 24 218 234 68 104 Ac. (3) 5400 2300 4600 2600 1900 2800 4700 3400 4300 3800 8700 1500 1900 1600 4500 700 800 2300 1500 1000 1400 800 1300 4200 1000 900 500 2600 500 800 400 124,100 LEN GTH Ft. CANAL Total-7 L-13L-10L-10 L-10L-12 L - 12L-14 L - 14L-14 L-14 L-14 L-14 L - 11L-14 L-14 L - 16L-17L - 17L - 18=Š. L-2 L-3 L-9 L-9 6-T

ENGINEERING AND DESIGN DATA Area 6 - Eadytown - Pineville - St. Stephen

Sheet 4 of 5	TO TAL	EST! MATED COST	Dollars (13)							11,581.00											16,260.00		()	2791.00										20.587.00			4557.00	700			
S		N EW	Length & Size (12)	30' - 54"	l I	15' U.T. Br.	1		1		40' - 30"	1	1 6	13. K.C. Br.	15' U.T. Br.				15' U.T. Br.	!			-		30' = 30"	1	1	Ω,	-	40.	i	1	1	30' - 24"		40' - 42"	-		40' - 24"	40' - 42"	I
		S L J	Ze	-	-	!	1	1			-		1	! !	1		-	-	;	!		20' - 15"	!		1			1	-	1 1		40' - 24"	!	!		!	1		40' - 18" 40' - 24"		
Stephien	REQUIRED	RT. OF WAY WIDTH	Ft. (10)	38	38	38	38	38	38		38	38	41	m m	38	38	38	38	38	38		38	38		38	000	46	46	46	ς, α. Σ α	38 8	38	38	38	38	38	38		38	38	
36.		γΑγ 4.G	Ac. (9)	4.8	2.1	4.3	0 0	1.8	1.2	17.1	1.8	2.2	7.00	7.0	1.4	0.5	ري د.	œ •	6.2	4.0	19.9	4 0	5. 0	4.6	4.0	i τ	. t. co	1 4	1.7	4 m	. t.	1.1	1.9	23 - 23 53 - 53	1.3	1.5	0 0 7	יָּי	1.5	1.1	
mevine -		z	Cu. Yds. (8)	9768	4292	8584	7104	2516	2516	34,780	3700	4440	4342	5476	2812	5772	4588	1628	2920	740	40,158	740	8436	97,16	888	3674	6936	3060	3672	9304	2220	2220	3848	2516	2664	3108	7252	TO.064	3108	2220	
Lauytown -	DIMENSIONS	AVERAGE DEP TH	Ft.	22	22	υ	ಬ	ω, i	ıO		22	Ω	ıO ı	o rc	υ Ω	22	ıç.	ιΩ :	ιΩ	ιΩ		ו מו	w		ιΟ п	οu	υ Ω	S	гОл	o 10	o ro	Ω	ın ı	ഹ	22	ιΩ	rs		ಬ	Ω	
ea o	<u></u>	BO TTOM WIDTH	, Ft. (6)	8	က	ന	က	က	n		m	m	4 (n 0.	ന	က	က	က	m	m		e .	m		m 0	2 4	• 9	9	0	D (**	ന	ო	n (n	m	က	m		m	m	
	CHANN	TOP WIDTH	Ft. (5)	13	13	13	13	13	13		13	13	44	L3	13	13	13	13	13	13		13	13		13) L	16	16	16	7 F	13	13	13	133	13	13	13		13	13	
		DISCHARGE	c. f. s. (4)	28	40	65	72	<u>ო</u>	m		6	20	040	0 K	64	15	11	15	58	28		2	000		0 0	7 7	r 00 -	96	00 7	110	2 4	Ю	11	4	15	25	40	ļ	17	57	
		WATERSHED	Ac.	372	556	932	1128	24	30		88	248	568	υ υ υ υ	1250	180	118	174	362	370		20	246		09	1150	1512	1580	1652	1980	0 00	48	120	09	178	306	560		88	370	
		LENGTH	Ft. (2)	0099	2900	2800	4800	1700	1700	23,500	2500	3000	2600	000	1900	3900	3100	1100	4000	200	26,800	200	5700	6200	000	00000	3400	1500	1800	4500	1500	1500	2600	1700	1800	2100	4900	2220	2100	1500	
		CANAL	, ox	M-8	M-8	M-8	M-8	L-1	L-2	Total-8	6-W	6-W	6 E	D 0	0 0 1 X	L-1	I-2	T-3	L-2	1-2	Total-9	M-10	M-10	Total-10	M-11.	3 7 7	M-11	M-11	M-11	IJ−TT-	1-1	L-2	L-2	L-3 Total-11	M-12	M-12	M-12	TOTAL-12	M-13	M-13	

ENGINEERING AND DESIGN DATA Area 6 - Eadytown - Pineville - St. Stephen

Sheet 5 of 5	TOTAL	ш				,	12,827.00		2196.00						9953.00	271,697.00	
		CULVERTS & BRIDGES - NEW	Length & Size (12)	50' - 36" 40' - 60'	15' R.C. Br.	1	1	-		20' - 48"	-	15' C.T. Br.	100' - 54" 15' R.C. Br.	j i	!		
		8 B	9 2	-	80' - 48"		ļ	40' - 30"		ope min	1	-		!	1		
Stephen	REQUIRED	RT. OF WAY	Ft. (10)	41	44	41	0	38	888	38	44	44	44	44			
200		¥ A¥ M G ¥	Ac. (9)	1.0	1.7	0 F	6.7	1.1	ഗ ധ	3.6	1.0	80 (0	o. N	O Ø	adequate 8.0	ဖ • ဗ ဗ	
Alea o - Lauytown - I meyine		EXCAVATION	Cu. Yds. (8)	2004	3700	0179	20,023	2220	4440	7252	2220	1665	4255	1295	is considered 16,687	715,303	
ran's roy	SIONS	AVERAGE DEP TH	Ft. (7)	ιΩ	ıΩı	υи)	ro.	ഹ	2	ιO	ιΩι	Ω	ιO	constructed		
Lea o	CHANNEL DIMENSIONS	MOTTOM WIDTH	Ft. (6)	4	ю.	4 0)	m	ო	8	ιΩ	TO I	ဂ	ις	g 0		
	CHAN	WIDTH	Ft. (5)	14	12	14	9	13	T3	13	15	12	12	15	nt canal		
		DISCHARGE	c. f. s. (4)	69	79	90 0	0	14	31	38	83	0000	102	112	Present		
		WATERSHED	Ac. (3)	466	550	678	027	68	176	228	568	808	740	832	252		
		LEN GTH	Ft. (2)	1200	2000	3700	12,400	1500	3000	4900	1200	006	2300	400	10,000	413,000	
		CANAL	No.	M-13	M-13	M-13	L-1 Total-13	M-14	M-14 Total-14	M-15	M-15	M-15	M-15	M-15	L-1 Total-15	Area 6 Grand Total	

Area 7 - St. Stephen - Alvin - Jamestown

Sheet | of 4 ESTIMATED 12,893.00 4759.00 4215.00 5869.00 4410.00 Dollars (13) TOTAL COST BRIDGES - NEW Length & Size CULVERTS & - 42" - 48" - 60" 24" 24" - 48" 30" 30 = 18. 18 36" 36" - 18" 48 54" - 36" (12) 1 404 30 . 30' 30' 30' 401 30 4 40 30 4 30. 30. 50 4 Length & Size CULVERTS LOWERING =1 1 1 1 1 1 1 1 REQUIRED RT. OF WAY WIDTH Ft. (10) 38 38 44 44 105 105 1124 1116 38 38 38 38 38 38 338 34 44 38 38 38 38 RT. OF WAY 0.7 4.0 6.4 0.7 1.2 2.9 4.8 2.9 1.3 0.9 1.7 0, 0, 10 0 0 0 1.9 4.4 1.4 13.8 0, Ac. (9) EXCAVATION Cu. Yds. 2072 6512 888 740 1332 2516 5920 1480 2664 2220 3404 1628 2072 3404 5624 3848 4292 9250 1332 2812 2516 3700 9620 6300 37,740 1332 1332 5920 2171 3060 4509 3374 12,876 9768 12,284 16,428 5624 11,248 33,129 17,125 3108 18,330 39,960 (8) AVERAGE DEP TH ئ. ك (7) CHANNEL DIMENSIONS 0000000 50 10 10 0 10 10 10 0 0 0 0 10 10 10 010101010101010 BOTT OM WIDTH (9) . نۍ خه **ო ო ო** 0000 n n n n m ന ന WIDTH T0 P ژپ سا (8) 13 13 13 13 13 13 13 13 13 13 13 13 13 13 13 13 13 13 13 DISCHARGE C. f. 8. 002600 15 333 233 7 38 27 47 67 67 102 113 11 18 40 23 43 69 81 130 177 400 462 472 535 WATERSHED 240 296 456 64 2088 116 208 552 296 1294 3340 10,370 40 166 346 246 268 452 92 352 348 1020 0407 2286 8982 56 96 100 64 78 88 15,058 Ac. (3) LEN GTH 900 4400 8700 1700 4000 2300 2300 3800 3800 7600 2900 1300 1500 5000 900 1900 00,400 1700 2500 2700 5200 1400 2000 3000 5400 5100 2500 900 500 4000 1400 Ft. L-1 Fotal-4 M-5 M-5 Total-5 Potal-2 Total-6 Total-1 CANAL M M L 2 M M - M 8 X X M-6 M-7 M-7 M-7 . Ç L-1M-7 M-7

ENGINEERING AND DESIGN DATA Area 7 · St. Stephen · Alvin · Jamestown

Sheet 2 of 4	TOTAL	EST! MATED COST	Dollars (13)																																									
		CULVERTS & BRIDGES - NEW	SIz ()	15' C.T. Br.		1	301 - 24"	I	1 1	15' C.T. Br.	[1	100	1	r,	1	-	!	١.	30' - 54"	Ů				15' C.T. Br.	1	1		-	15' C.T. Br.				15 ° C.T. Br.	1		30' - 30"	15' U.T. Br.	U.T. Br		15' U.T. Br.	1	1	30' U.T. Br.
		CULVERTS	Length & Size		Ì I	-	-		ł	-	1	1	1		-		1	-	1	!		-	1	1	-				!	1				1	1	1.		1		į.	!	-		
CAATI	REQUIR	RT. OF WAY	Ft. (10)	38	44	38	20 00	0 0	0 00	38	38	38	89 88 87 88	0 88	38	44	55	68	94	38	44	44	38	38	41	44	D 00) e	38	44	44	m 0	D & &	38	38	80 00	m m) @	88	38	46	46	49	57
CALVALL GOLDSCOLOFF		CLEARING	Ac. (9)	2.9	1 .2	2.1	χ, _τ	T. C	. O.	3 .7	1.6	1.1	4.0	3 00	ω • • • •	2 .7	1.7	4.4	ກຸດ	ກ•1 <	. t.	0.7	1 .4	1.2	4 °	I.I	π - c	+ o.° O	2.6	5.4	1.7	ဘာ ၈ လ ၈	ນ T ນີ້ ໜີ	3.1	o. O	03 (0 00 -	4 0	0.0	3.2	1.1	2.7	1 .4	က္	4.8
OLCHICIE - FAIVE		EXCAVATION	Cu. Yde. (8)	5772	2338	4144	2624	07.10 07.18	444	7548	3256	2220	4884	4144	7849	5735	3885	10,912	14,058	2004	8660	1480	2812	2516	9018	2405	3700	1776	5180	11,470	3515	5772	3108	6216	1776	4588	740	3996	6512	2220	5916	3060	7326	11,398
Topic office	UINENSIGNS	AVERAGE DEPTH	(7)	2	ا (کا	IO I	ОП	ט ע	о 10	D.	D	D	n n	э rc	ω Ω	Ю	ro O	IO I	Ω F	υи	ω ω	D	Ю	Ŋ	Юп	Ωи	ח מ	о го	Ю	Ю	ro i	υи	о О	S	ಬ	ا مَا	n n) IC	Ŋ	Ŋ	ις	l CI	S	N
	4	MO TTO W WIDTN	Ft. (6)	က	4	ന (n 0	n 0	ാത	თ	თ	თ -	നെ	ാന	4	ъ	o o	14	18	n <	r LO	IJ	ന	ന	4 F	വ	n 0	ാ ന	က	ro	ro ·	n 0	ာက	თ	က	ന (m m) m	m	ć	9	0	4	10
7	CHANN	WIDTH	Ft. (5)	13	14	13	n c	7 70	T F	13	13	13	133	T F	14	15	19	24	5 88	μ Σ -	15	15	13	13	14	T T	T T	H H	13	15	15	T T T	T F	13	13	13	- T	1 1	13	13	16	16	17	000
		DISCHARGE	c. f. s. (4)	46	89	25.5	0 5	T T	% F G	29	62	13	11	-1 00 0 00) (C)	94	140	208	271	7 6	78	64	18	35	73	2,00	10	# 00 H	40	78	88	ري د د	Ω Η	40	42	13	0000	20 02	65	75	146	151	160	213
		WATERSHED	Ac. (3)	658	952	312	252	140	368	876	940	140	124	4 400 800 800 800 800 800 800 800 800 800	956	1208	2484	4040	5510	310	1226	1242	210	480	1140	1204	160	80	552	1234	1306	320	80	556	588	156	56	720	984	1164	2632	2728	2932	4120
		LENGTR	Ft. (2)	3900	1400	2800	3800	1700	300	5100	2200	1500	3300	4300	4700	3100	1500	3100	3300	1800 1800	3600	800	1900	1700	5400	1300	0000	1200	3500	6200	1900	3900	2100	4200	1200	3100	2000	2000	4400	1500	2900	1500	3300	4100
		CANAL	No.	L-1	L-1	L-2	L-3	L-4	1 1 4		L-4	L-5	I-6	1-1	11-7	L-7	L-7	L-7	L-7	1 - 3	0 0	L-8	6-1	1-9	L-9	0-1 1-0	L-TO	1-12	L-12	L-12	L-12	L-13	L-14	L-15	L-15	L-16	1-17	1,-17	L-17	L-17	L-17	L-17	L-17	L-17

ENGINEERING AND DESIGN DATA Area 7 - St. Stephen - Alvin - Jamestown

Sheet 3 of 4	TOTAL	ESTIMATED COST	Dollars (13)										141,906.00														_				-														
Sh			size 2)		30' - 30"	o T.		30' - 54"	1	-	-	40' - 24"		40' - 42"	-			-		-	! !		!	-	I	30' - 36"	!			15' U.T. Br.		C.T.	15' C.T. Br.	- C - K-	C.T.		30' - 60"		3-15' U.T. Brs	1	-		1	1	ì
		CULVERTS	Length & Size (11)	-	1	l I		1	1	-	-	-	1			1	1	1	1	-		!		-	50' - 24"		-			-	1	ļ	-	!		-		-		-		-	1		
	REQUIRED	RT. OF WAY	Ft. (10)	38	82 00	0 0	38	38	38	38	38	38	200	38	38	52	22	78	105	, eg	110	116	38	38	38		46	С V П п	73	41	46	38	0 r	00 W	73	38	38	38	44	46	38	44	44	38	38
2011		RT. OF WAY	Ac. (9)	2.7	٦ °	. c	, O	. t. co	1.1	1.8	0.4	0 ° °	189.0	2.1	0.8	2.5	4°.3	T. 6	T . €	. i	10.3	0.00	0.0	4.1	4.3		6.4	4. c	5 . V	2.4	3.8	8.4	n 01	ο α Λ 4	7.7	2.4	8.0	3.8	3.7	1.6	3.7	8.8	0,0	1.2	₹.
		EXCAVATION	Cu. Yds. (8)	5476	2860	1007	1036	6216	2220	3552	888	5180	424,165	4292	3996	5784	9842	23,430	3000	16,000 900F	8503 87,864	37,675	5920	8288	8584		14,280	9399 5180	18,672	5010	8364	9768	11,832	11,814 20 768	19,450	4736	5180	6364	7770	3468	7400	5920	4625	2516	2516
	SIONS	AVERAGE DEP TH	Ft. (7)	ıΩı	οи) LC) IO	D	Ю	Ŋ	Ω	ıo ı	n	D	ıO	ro	ω ·	ıo ı	O F	Ω	ט נט) IQ	22	2	2	1	ا م	ט נט	о го	rs	2	D	ro n	ט וכ) IO	Ŋ	5	2	Ŋ	Ω	D.	D.	ıΩı	5	ro ——
	EL DIMENSIONS	BOTTOM WIDTH	Ft. (6)	e (n 0	ე ო) m	m	m	က	ന	ကေးင	n	8	ო	00	o :	18	0 0	2 22	30	00 00	, m	က	ന	,	·O (ာ တ	16	4.	0	က	w c	D 7	16	ෆ	က	ന	Ŋ	9	m	Ŋ	D O	m	m
	CHANN	WIDTH	Ft. (5)	13	T π	o c.	13	13	13	13	13	133	7	13	13	18	6	888	D C	ν ς γ ς	4 4	42	13	13	13	,	9 0	9 0	28	14	16	13	16	L 0	26	13	13	13	15	16	13	15	15	13	133
		DISCHARGE	C. f. s. (#)	24	n 0 4	r C	80	28	31	ιΩ	14	40	0	21	27	88	105	263	X 0000	0.00 0.00	57 CO	568	27	36	27	1	7.5	100t	168	45	99	48	000	157	171	83	33	22	81	06	35	75	080	27	TT
		WATERSHED	Ac. (3)	300	000	1980	1288	364	416	46	160	564	OTO	254	362	1424	1764	5256	SOLO	87.28	11.882	13,454	360	492	336	(1188	1868	3076	648	1008	592	1016	2790	3120	292	440	848	1298	1458	480	1168	1288	340	120
		LEN GTH	Ft. (2)	3700	2000	1,000	002	4200	1500	2400	009	3500	196,400	2900	2700	2400	3800	5500	009	3200	4300	5500	4000	2600	5800	(7000	0000	4800	3000	4100	0099	5800	4000	2000	3200	3500	4300	4200	1700	2000	3200	2500	1700	1700
		CANAL	. O.N.	L-18	1 20	1 20	1-20	1-21	1-21	L-22	L-23	1-23	Total-7	M-8	M-8	M-8	W-8	M-8	0 C	0 0 E Z	0 00	W-8	. T-1	L-2	L-3		L L	n m	L	L-4	L-4	L-5	. L-3) L	L-5	L-6	L-7	L-7	L-7	L-7	L-8	L-8	1 - 8	L-9	L-10

ENGINEERING AND DESIGN DATA Area 7 - St. Stephen - Alvin - Jamestown

Sheet 4 of 4 ESTIMATED 116,167.00 15,283.00 308,554.00 Dollars TO TAL COST (13) CULVERTS & BRIDGES - NEW Length & Size - 24" - 54" - 42" (12) 30 1 40, CULVERTS LOWERING Length & Size | | | | REQUIRED RT. OF WAY WIDTH Ft. (10) 38 38 38 38 RT. OF WAY CLEARING adequate (6) 1.6 1.6 Ac. 417.3 considered EXCAVATION Cu. Yds. 5328 2516 9686 3256 3256 2072 3404 6105 5328 5032 10,508 7326 3700 936,793 370,164 (8) 1.5 AVERAGE constructed Ft. DEP TH CHANNEL DIMENSIONS 20 20 20 0 0 0 0 0 0 0 0 0 0 BO TTOM WIDTH (9) د قبر سفا 0 0 4 0 ය ග canal TOPWIDTH Ft. (5) 13 13 14 13 13 13 13 13 13 13 Present DISCHARGE c. f. s. (4) 23 411 72 98 135 18 33 27 40 20 26 66 10 WATERSHED 240 332 1012 100 2400 432 348 560 1124 Ac. (3) 5800 2200 2200 1400 2300 3300 3300 3600 3400 3600 1700 144,600 7100 432,800 LEN GTH Ft. (2) Total-9 Total-8 CARAL M - 10L-12L-11 L-11 L-11Area 7 . . . Grand Total

ENGINEERING AND DESIGN DATA Area 8 - Russellville - Bonneau - Macedonia

Sheet 1 of 5	TOTAL	ESTIMATED COST	Dollars (13)																			•				_															-			
		CULVERTS & BRIDGES - NEW	Length & Size (12)	-	Ì	40' - 54"		-	!	1	1				-		(70. K.C. Br.	1	-		1	ł	1	30' - 36"	1		40' - 36"		15' U.T. Br.	40' - 48"	1	30' - 54"	-	1	15' U.T. Br.	-	30' - 48"	30' - 48"	50' - 24"		201 - 18"	I	15' U.T. Br.
		CULVERTS	Length & Size (11)	80* - 24"	-	!		1		1	1	1 1	!	-	1	-	!			i.	-	-	i	40' - 24"	!			-	-	-	!	1	!	1	1	-	ļ	-		1	1		!	
Macedonia	REQUIRED	RT. OF WAY WEDTH	ft. (10)	38	38	8 8 8 8	9 4	52	62	62	73	n 60	68	124	124	150	190	190	190	190	190	244	244	38	38	38	00 00 00 00	38	38	38	38	38	38	38	38	38	1 1 1	38		38	38	38	0 0	38 8
		RT. OF WAY	Ac. (9)	1.2	δ, 4,	L . 4	N. CO.	2,3	3.9	8.0	3.7	4 O	, e	14 .1	6.4	8 6	თ (1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	4 5	13.9	17.7	18.0	26.2	1.1	က ·	1.4	 		0.5	1.5	٥٠ ١٥	1,1	ري د.	3.7	1.8	0,10	3.1 adequate	25.23		2.4	1.0	6, -	⊢ . ∠	0.0
lle - Bonneau		EXCAVATION	Cu. Yds. (8)	2516	10,952	2812 1036	12,240	5302	9450	1890	9336	10,892	10,000	38,480	13,320	24,076	26,483	54,810	12.040	39,732	50,568	51,942	75,552	2220	6660	2812	0000	5476	1036	2960	5476	2220	4588	7548	3552	1924	s considered	4440		4736	2072	3848	OKTO	740
Russellville	DIMENSIONS	AVERAGE DEP TH	Ft. (7)	5	10 H	0 10	20 (0	Ŋ	ιςı	rs.	ເດັນ	ຄຸດ	Ω (ß	ro.	۱ م	n u	מו כ	വ	CZ CZ	Ω	ß	ß	ı مı	ı ما	n n	מ נת	വ	ß	ಬ	Ŋ	Ŋ	22	വ	ıo .	n u	tru	2		ß	Ŋ	ເດເ	o u	Ω Ω
- 0 a	<u></u>	BOTTOH WIDTH	Ft. (6)	n	ന (n m) ©	00	12	12	1.6	0 K	223	35	35	45	20 00		000	09	90	80	80	m (m (ကင	n m	n m	m	m	ო	m	ന	m	m ·	m <	as	n		თ	n	m (n 0	n m
Are	CHAKN	TOPWIDTH	Ft. (5)	13	130	- F	16	18	22	22	28	N W.	3 8	45	45	20	2 2	2 2	70	200	70	06	06	13	T3	T3	. t	13	13	13	13	13	13	13	133	13	sent cana			13	13	13	7 F	- F
		DISCHARGE	c. f. s. (4)	6	19	4 K	88	124	178	179	230	329	332	512	530	657	808	970	984	1024	1042	1272	1287	o ;	2000	N 1	T (C)	14	15	46	58	32	29	59	0	0 0 0	e.			34	36	13	O.L	54 50
		WATERSKED	Ac. (3)	06	234	084 730	1422	2142	3310	3324	4524	4304 6898	6968	12,264	12,336	15,874	20,322	64, 884	25,700	26,748	27,784	34,628	35,774	9.6	340	H 00	2 C C C C C C C C C C C C C C C C C C C	166	176	652	364	420	376	884	96	428	296	312		460	488	152	414	808
		LEN GTH	Ft. (2)	1700	7400	0061	0009	2200	3000	009	2400	2800	2000	5200	1800	2600	2200	7300	1000	3300	4300	3300	4800	1500	4500	1900	000%	3700	400	2000	3700	1500	3100	5100	2400	1300	2000	3000		3200	1400	2600	4800	500
		CANAL	No.	M-1	M-1	Z; Z	M-1	M-1	M-1	M-1	M .	T - X	M-1	M-1	M-1	M-1	M-1	H-E	i et E M	M-1	M-1	M-1	M-1	L-1	[-1	7 - 1	2	L-4	L-4	L-4	L-5	L-5	L-6	L-6	L-7	L-7	L-8	1-9		6-7	6-T	L-10	η-T-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-	L-11

ENGINEERING AND DESIGN DATA Area 8 - Russellville - Bonneau - Macedonia

Sheet 2 of 5 ESTIMATED Dollars TOTAL CO ST (13) BRIDGES - NEW Length & Size Br. Br U.T. Br. Br CULVERTS & - 36" - 24" - 36" - 48" - 60" - 42" - 30" R. C. R.C. U.T. 40 t 301 30 1 201 404 40 151 40, 15, 15, 15' Length & Size CULVERTS - 48" - 36" 80, 404 REQUIRED RT. OF WAY WIDTH Ft. (10) $\begin{array}{c} 144 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 338 \\ 348 \\$ 44 38 38 44 46 440 440 440 OF WAY CLEARING adequate 0.00 8.3 1.4 4 S 1 · 3 2 · 7 1 · 0 1 · 0 2 · 0 2 · 0 0.0 4. 8. 8. 4. 0. 0. 5.4 1 55 Ac. (9) considered EXCAVATION Cu. Yds. 6475 5845 2442 2886 5698 2516 8820 5920 1169 4579 4995 5916 1836 3006 3996 4292 6216 4440 4440 2812 7400 3848 5180 1332 2960 2664 5476 1924 1924 2960 3404 4884 1036 1776 5772 2368 15,030 2072 1632 4662 5920 11,189 501 20,726 19,839 16,206 14,652 (8) AVERAGE Ft. (7) DEP TH CHANNEL DIMENSIONS BO TTOM WIDTH ژ. ك (9) canal WIDTH T0 P <u>ئ</u>ـ (5) 14 113 113 113 113 113 19 22 114 113 113 115 116 117 119 119 DISCHARGE c. f. s. (4) 137 183 19 29 32 237 83 53 40 43 50 10 41 50 87 66 105 WATERSHED 610 720 1034 1074 1322 340 560 446 2310 100 250 274 976 570 3448 4736 254 224 342 148 100 2234 1106 730 1384 1416 1652 1768 1920 2552 146 780 102 2430 Ac. (3) LEN GTH 2900 3000 3000 1700 7300 2500 2800 6700 1900 5000 2600 3500 900 2000 1800 3700 1300 1300 2000 4000 2300 3300 700 300 8600 1900 1200 3900 1600 9000 3500 1400 800 2900 1100 1800 2200 Ft. (2) 1,-24 L-16 L-16 L-18 L-18 222 L-28 L-28 L-28 L-28 L-12 L - 16L-17L-18 L-19 1-20 1-21 6-21 1-22 1-22 2-23 1-24 L-24 1-24 1-24 1-25 6-27 L-27 1-28 L-28 L-28 L-28 L-28 L-28 L-28 CANAL No.

DATA	,
DESIGN	li p
AND D	
	THE ARE
ENGINEERING	4
Ш	4

ESTIMATED Dollars (13) Sheet 3 of COST TO TAL BRIDGES - NEW Length & Size Br. Br. Br. Br. Br. Br. R.C. Br. Br Br CULVERTS & 15' U.T. Br. - 54" 54" 42" - 48" - 30" 30' - 48" C.T. R.C. R.C. U.T. C_{\bullet} $\mathbb{T}_{L_{\bullet}}$ U T. (112) R. C. U.T. R. C. 404 301 304 404 401 15' 301 15' 30, 30 4 15' 15' Length & Size CULVERTS - 36" =80. REQUIRED RT. OF WAY WIDTH Ft. Area 8 - Russellville - Bonneau - Macedonia 41 46 38 46 RT. OF WAY (6) 1.2 5.0 10.5 2.1 6.2 3.1 4.3 6.3 4.1 4.4 1.9 2 .1 .6 2.1 4.8 7.9 7.0 1.7 2.0 3.2 1.0 3.7 1.6 1.3 1.3 EXCAVATION Cu. Yds. 2368 11,322 4292 15,949 10,503 2516 2072 5772 8851 7140 1924 14,076 8547 9730 26,048 3404 3848 2516 4144 3256 4144 3256 12,859 6993 7560 3150 3168 7000 8140 3404 592 2368 3108 2664 1924 6364 4440 6808 4342 6528 1020 4692 10,912 444 10,952 (8) AVERAGE D EP TH Į, (7) CHANNEL DIMENSIONS BO TTOM WIDTH نډ خا 9 TOP WIDTH (5) 16 DISCHARGE c. f.s. (4) 233 196 167 203 310 33 38 50 36 160 29 52 15 167 47 WATERSHED 2076 2932 4122 3048 3878 4596 6396 448 152 40 96 488 1188 1508 268 1544 2696 3746 86 156 384 344 656 976 2544 3090 860 432 532 672 732 752 256 82 836 1144 584 Ac. (3) 3300 7400 3100 2300 2600 2900 2800 300 2800 2200 2700 2400 1000 2300 1600 1800 1300 1600 5100 3500 1700 900 4100 1400 5500 7400 400 4300 3000 2000 3900 5300 3500 6900 4600 2600 2600 1700 1400 1300 LEN GTH Ft. (2) CANAL L-35 L-35 L-36 L-36 L-36 L-36 L-36 L-39 L-40 L-41 L-42 L-46 L-47 L-48 L-33 L-34 L-35 L-36 L-36 L-38 L-39 L-41 L-48 L-32 L-37 L-41 L - 41šĘ

ENGINEERING AND DESIGN DATA

376,446.00 71,959.00 ESTIMATED Sheet 4 of Dollars TO TAL (13) COST BRIDGES - NEW Length & Size 15' U.T. Br. CULVERTS & C.T. Br. Br R.C. Br. 30" - 30" - 42" - 30." 60' - 48" - 24" - 30" - 54" - 30" - 48" (12)R. C. 1 40' 40, 30 : 40. 501 40 50 1 40 4 151 15, 15' Length & Size CULVERTS LOWERING - 30 1 1 50 t RT. OF WAY REQUIRED WIDTE Ft. (10) Area 8 - Russellville - Bonneau - Macedonia 38 38 38 RT. OF WAY adequate adequate adequate CL EARING 1.0 2.9 2.6 490.6 (8) 0.4 5.7 3.8 0.0 1.1 Ac. 3.4 4.6 4.0 9.5 1.5 1.8 2.7 2.1 8 2.7 2.6 2.1 1.2 1.2 as constructed is considered as constructed is considered considered EXCAVATION 8190 2072 5920 5328 11,544 4410 3150 8947 4686 8334 12,000 10,500 2516 7696 8350 3108 3552 5476 4292 5920 5476 5180 4292 1776 8436 2220 2368 740 1924 2220 4440 3552 12,837 8797 1,166,684 25,239 7871 216,551 Cu. Yds. (8) i.s constructed BOTTOM AVERAGE Ft. (7) DEP TH CHANNEL DIMENSIONS 0 0 0 5 $\hat{\boldsymbol{\omega}}$ $\boldsymbol{\alpha}$ $\boldsymbol{\alpha}$ WIDTH (9) 4.1 14. m ന ന ന g 8 112 112 113 116 116 116 20 20 20 22 22 22 24 24 Present canal Present canal canal WIDTH T0 P (2) . نيا سنا 13 13 13 Present DISCHARGE c. f. s. (4) 181 292 322 327 353 14 30 53 39 282 WATERSHED 388 540 2986 3398 4388 140 1140 110 1702 5444 5728 6038 6132 6656 6860 7256 546 234 432 160 144 710 176 296 100 394 Ac. (3) 4776 236 58 252 422 3600 4000 1400 2300 1100 1700 1800 2400 2100 4700 1700 5200 5000 2100 2400 3700 2900 4000 3700 3500 2900 1200 5700 1500 500 1300 3000 1700 1600 1500 2400 LENGTH 94,300 Ft. Total-2 Total-1 CARAL L-49 L-49 (M-2 1-5 L-8 L-9 L-9 L-2 0 L-1

DATA	Macadania
DESIGN	1
AND	Lo Ronnogu
ENGINEERING	Ruccollyillo
山乙コワフ	2 8 -
<u>П</u>	~

Sheet 5 of 5	TOTAL	ESTIMATED COST	Dollars (13)				13,225.00			7110 06	000			0	3449.00	469,186.00									
			Length & Size (12)		1	9601 194"	ı		60' - 30"	-				!											
		CULVERTS	Length & Size (11)		401 - 60"	1			-			1 !	1	-											
Macedonia	REQUIRED	WAY	Ft. (10)	38	8 (8	ж ж «	o o	38	38	88	000	0 8	38	38											
		N A Y	Ac. (9)	1.3	ω i	, o	8.9	1.5	8.8	η C		 	1.7	0.1	/.· C	605.7									
Area 8 - Russellville - Bonneau		×	Cu. Yds. (8)	2664	5624	5476	17,908	2960	5624	3108	14000	3108	3404	3552	77,044	1,424,379									
Russellvi	DIMENSIONS	AVERAGE DEP TH	Ft. (7)	2	ıo ı	o r)	2	ທ	ιΩ	n	טו כ	10	ស											
ea 8 - 1		80 TTOM WIDTH	Ft. (6)	ю	m c	יה מי	0	m	en -	m	0	n c	(n)	е						 	 -	 			
Ar	CHANNEL	WIDTH	Ft. (5)	13	13	13	O H	13	13	e H	0	- F	13	13					 	 	 			 	
		DISCHARGE	c. f. s. (4)	15	N 00 0	 	1, O	6	18	CS CS		1 1	000	12				 		••••					
		WATERSHED	Ac. (3)	176	8 1 8 1	540	OTO	06	216	068	0	180	432	138						 	 				
		LEN GTH	Ft. (2)	1800	3800	3700	12,100	2000	3800	2100	7	P 100	2300	2400	7800	543,300									
		CANAL	No.	M-3	M-3	თ ი Σ	M-3 Total-3	M-4	M-4	M-4	10001-4	0 K 1 1 2 Z	M = M = M = M = M = M = M = M = M = M =		Total-5	Area 8 Grand Total									

ENGINEERING AND DESIGN DATA Area 9 - Macedonia - Bethera - Gough

Sheet 1 of 4	TOTAL	ESTIMATED COST	Dollars (13)					-													-			•											•										7 L L	77. • 07. •
		CULVERTS & BRIDGES - NEW	~ ~		15' C.T. Br.	-	1	15' C.T. Br.	1	-	-	-	1		1	1	-	!	-	1 (15' C.T. Br.	!	30' - 54"	i	-	15' C.T. Br.	i	30, - 60,		Į.	15' C.T. Br.	1	6	LO C.T. Br.	-	-	1	15' R.C. Br.	1	-	-		i	40' - 42"	:	
		CULVERTS	Length & Size (11)	-	-	-	1	1		-	-	-	!		-	-	-	!	1	40" - 48"		1	-	-	1	-	1	-	! !	ŀ	1	-				1	-	}	-	1	-	!	-	1	!	
	REQUIRED	RT. OF WAY	Ft. (10)	38	38	38	44	52	52	78	78	00 C	, C	137	124	137	150	38	38	38	8 6	38	38 38	38	38	38	41	33	55 C	38	46	46	38	38	38	41	38	49	49	55	38	38	38	33	95	
Demera - Oough		RT. OF WAY	Ac. (9)	2.9	0.5	1.6	ന യ	5.0	2° (S	တ္ (1.7	ω c	א מ	11.1	21.1	13.2	11.9	4.8	1 8	1.2	٠° د د د د د د د د د د د د د د د د د د د		5. 2	3.1	2.5	4.8	्र १ १	0 10	0 0	φ. 4	4.1	0.0	1.8	, w	1 0	7.4	5.1	3.7	1.0	6.2	3.4	1.4	6. 4 6. 1	n 0	7 · T	0.400
		EXCAVATION	Cu. Yds. (8)	5920	1036	3256	8140	11,327	7230	17,466	4260	23,000	14 904	30,821	57,720	36,652	33,336	9768	3700	2368	7400	7488	11,544	6216	4440	9768	4509	10,064	LU36	8584	9180	2040	3552	0328	3256	15,364	10,360	8214	2220	14,245	8089	2812	9324	5032	0102	404,004
	SIONS	AVERAGE DEP TH	(2) Et.	2	22	ಬ	ro C	ro.	ı Oı	ı n	υı	n n	ט ע	о го	ಬ	2	S	ro	ro	i OI	U H	רו כי	വ	2	2	5	i Oi	ω n	0 10	വ	2	20	ro r	n in	2	22	Ŋ	D.	ಬ	വ	Ŋ	ı ك	ıΩ	ע מ	Ċ	
Tri cm	CHANNEL DIMENSIONS	BO T T O W	Ft. (6)	, co	m	ო	Ŋ	Φ	ω (18	18	23 5	† C	40	35	40	45	ო	ო	က (n c	ი «	· თ	, m	က	ო	4 (ကျ	ν α) ന	9	9	ကပ	n <	· რ	4	က	7	۲.	o	ო	က	ကဖ	က က	'n	
	CHANN	TOP	Ft. (5)	13	13	13	15	18	18	28	28	325	# CV	200	45	20	55	13	13	13	T3	16	13	13	13	13	14	133	18	13	16	16	13	14	13	14	13	17	17	19	13	13	13	T T	ΤC	
		DISCHARGE	c. f. s. (4)	20	54	09	84	113	118	223	231	2000	n 00 00	490	518	539	631	37	o	11	35	24°C	0000	09	12	90	99	S C	20°L	01	88	86	27	000	11	71	51	111	112	141	20	09	41	16	0	
		WATERSHED	Ac. (3)	250	794	894	1346	1902	2042	4322	4472	5874	# W	11,202	12,062	12,554	15,248	504	06	116	492	1700	380	006	130	006	1006	426	4.00 ひ R 00	920	1448	1632	358	7.30	112	1106	750	1872	1912	2514	252	910	582	194	242	
		LEN GTH	Ft. (2)	4000	700	2200	4400	4700	3000	4100	1000	4600	2300	3700	7800	4400	3600	6600	2500	1600	2000	2200	7800	4200	3000	0099	2700	0800	00%	5800	4500	1000	2400	3800	2200	9200	7000	3700	1000	5500	4600	1900	6300	3400	1700	F/3,000
		CANAL	No.	M-1	M-1	M-1	M-1	M-1	M-1	M-1	M-1	M-1	T ← Z	Σ Σ	M−1	M-1	M-1	L-1	2-7	L-3	E - C	. L	1-4	L-4	L-5	L-6	L-6	L-7	17	L-8	L-8	L-8	6-7	n o	L-10	L-11	L-12	L-12	L-12	L-12	L-13	L-13	L-14	L-15	L-LO	T - TD 0 OT

DATA	ıgh
DESIGN	ethera - Go
AND	donia - B
EERING	9 - Mace
ENGIN	Area

Sheet 2 of 4	TOTAL	COST DOIL ALA	(13)																																				00 808 80	TO3, 303,00			
		CULVERTS & BRIDGES - NEW Length & Size	2)		1 1		-		-	1	-			15' C.T. Br.			1	i	15' C.T. Br.		i 1	i) - GC	15' C.T. Br.	15' C.T. Br.		-	30' - 24"	1		15' C.T. Br.		1	0000		30' - 36"	- 1	ŀ			40' - 30"	-	1 1
		CULVERTS LOWERING	, <u> </u>		1 1	1		-	1	-	1	! !	1	-	1	1			1	-	1	1		-	 1	1	1	-			1		1	-			I	-			40' - 36"	-	I
	REQUIRED	MIDTH Ft.	(01)	38	41	025	78	78	94	66	105	110	110	38	46	46 98	0 00	0 00	38	38	41	44	38	00 ⁷	44	46	46	38	33 G	0 00	38	38	38	m 0	0 00	38 8	38	38	38		38	38	38
Maccoonia - Demera - Cougn	2	- O	(6)		1.4	5.7	7.1	e . e	7.5	1.5	13.9	0 0	14.8	4.2	4.9	N -	1. ←	0.4	4.7	2.9	3.9	4.4	0,0	ы 100 г	0. W	1 t	1.7	2.5	O () m	H (S)	0.3	8.0	w +	D - C	· O ·	1.7	1.1	3.57	141.0	4.7	3,8	1.0
onia - Deti		Cu. Yds.	(8)	6956	3006	13,860	18,318	8520	19,869	4018	37,271	1296	40,176	8436	10,812	0.0200 30.0200	0020	3848	9472	5772	8183	9250	5772	3700	8160	3264	3876	2368	888	6216	2368	1036	1628	9980	1480	2072	3404	2220	7104	320,200	9472	7696	4144
- Maccu	SMOIS	AVEKAGE DEPTH Ft.	(7)	ហេ ព) IO	Ŋ	ſΩ	Ŋ	D	Ŋ	ស ៣	о го	N	Ю	ro I	O 10) IC	ω Ω	22	D	23	D	ı, ı	n n	о О	10	ro	ın ı	n u	n 10	ß	ro	വ	U R	ט וכ	Ω (S)	10	ro		Ŋ	ro	വവ
Area y	EL DINEN		(9)	თ ₹	1 4	12	18	18	24	26	288	000	30	e	0	:o r	o er.) m	m	က	4	ro.	က (ന പ	ာဖ	w w	9	m (n c	ാന	n m	m	n	n m	o m) m	ಣ	က	က		n	ന	w 4
2 100	CHARN	WIDTH Ft.	(2)	13	14	22	28	28	34	36	38	40	40	13	16	13	13	13	13	13	14	15	13	n ⊩	10	16	16	13	H -	n H	13	13	13	T 3	- H	13	13	13	13		π E	13	13
	100430010	C.f.s.	(†)	34	73	177	255	260	343	380	402	43.5	446	42	35	30	20 00	. C	30	47	67	78	35	44	, a	06	66	φ (D (3 4 3 W	53	60	0	200	. 4	13	23	26	23		23	44	50
	0 0 0	WA LEROHEU AC.	(3)	464	1146	3278	5096	5276	7316	8238	8862	9598	10,018	594	1362	1728	00° €	126	480	999	1028	1238	422	0220	1444	1484	1628	58	m c	810	068	808	74	240	H C	144	288	332	296		328	628	722 1128
	a Form	Ft.	(2)	4700	1800	4400	4300	2000	3700	400	6100	200	6200	5700	5300	00000	0000	2800	6400	3900	4900	2000	3900	8200	4000	1600	1900	1600	2000	4400	1600	700	1100	4500	1000	1400	2300	1500	4800	133,800	6400	5200	2800
	2	NO.	(=)	Z Z	2 00 I I ⊠	⊇ 23 - W -	M-2	M-2	M-2	M-2	W X	ν α 	Z - X	1-1	L-1	L - L	\$ [] [. n	1-4	L-4	L-4	L-4	L-5	L-5	L L L	L-5	1,-5	1-6	1 P	7-7	1-7	L-7	L-8	- L-0		1-11	L-11	L-11	L-12	Z-TEIOL	M-3	M-3	M-3

ENGINEERING AND DESIGN DATA Area 9 - Macedonia - Bethera - Gough

Sheet 3 of 4	TOTAL	ESTIMATED COST	Dollars (13)		10,747.00										10,375.00												_																0 0 0
		CULVERTS & BRIDGES - NEW	Length & Size (12)		1			30' - 54"	40' - 48"		1		40' - 30"	-		15' C.T. Br.	1		-	I	30' C.T. Br.	Į į	1	30' - 30"	ì	40' - 36"	1	1	1	1	i	30' - 30"	i	-	T	40' - 30"	1	15' C.T. Br.		1	! !	1	30" - 30"
		CULVERTS	Length & Size (11)	1	!!		1	-	40' - 30"	1 1		I 1	-	-			1	! !	-	1	!	-	!		ļ	401 - 36"		!		1	-	401 - 24"	i		-	!	-	-	1			1	}
u.o.	REQUIRED	RT. OF WAY WIDTH	Ft. (10)	38	38	38	38	38	38	38	38	0 00	9 (8	38		38	2 8	94	49	73	78	00 0 4 0	n a	88	38	38	38	46	55	55	38	n c	0 00	41	38	38	38	38	80 (0	0 7 2 7	+ aa † m	38	38
era - con		RT. OF WAY	Ac. (9)	2.8	1.1	3.51	0.7	0.4	0.7	1.0	w 0	, r.	. T	1.0	13.6	000	, v <		1.4	1 .7	2.0	10.4	w c	. 0	2.4	2,5	1.1	. 8	1.3	2.7	4.00	4.0	⊋ O.	7.6	1.7	0.4	0.7	1.3	. t		1.6	' ଓ	8 6
- Maccuonia - Demera - Cougn		EXCAVATION	Cu. Yds. (8)	5624	2220 33,164	7104	1480	740	1480	2072	5328	1400 000 000 000 000	2960	1924	27,528	12,284	10 200	3264	3108	4279	5112	26,854	7180	1628	4736	2516	8346	6324	3108	6216	6808	047	5772	15,698	3404	888	1332	2664	2516	103h	2812	5328	5624
Maccu	SIONS	AVERAGE DEP TH	Ft. (7)	22	Ŋ	23	10	Ю	Ю	10 1	Юп	ט וכ	n 10	Ŋ		ro r	οк	າທ	ro	Ю	Ю	Юп	ט ני	о го	ю	10 1	ממ	ω N	Ю	Ŋ	ນ ເ	ט זע) IO	D	Ŋ	D.	ro.	ا ي	ıo u	ρи	ט וכ	വ	Ŋ
Area 7	L DINEN	Z	Ft. (6)	8	n	co	· m	က	က	en (თ ი	ი ო	n m	က		(m)	n (0 0	7	16	18	0 0	2 0) m	က	m (n ⊲	1 0	0	o	m (ာ က	ာက	4	က	က	က	က	თ ი	∞ <	r m	ာက	က
	CHAKN	TOP	Ft. (5)	13	13	13	13	13	13	13	13	T F	13	13		13	± 4.5 β.4	16	17	26	28	30	30	H H	13	13	L3	16	19	19	13	T -	13	14	13	13	13	13	13	L3	13.4	13	13
		DISCHARGE	c. f. s. (4)	20	23.4	20	22	30	31	34	54	യ	10	12		88 1	0 a	105	110	229	272	294	177	i ©	19	000	N W	88	141	146	0 0 1	11	225	99	15	24	30	44	ф Ю	D 0	y 00	13	11
		WATERSHED	Ac. (3)	248	304	252	328	388	418	452	796	500	104	134		524	7.04	1766	1856	4516	5532	6062	0460	48	228	248	1018	1458	2508	2592	540	118	318	1000	172	302	402	628	708	0 0 0 0 0 0	##6 080	150	116
			Ft. (2)	3800	1500	4800	1000	200	1000	1400	3800	0000	2000	1300	18,600	8300	3000	1600	1400	1100	1200	5800	20084 00084	1100	3200	1700	0000	3100	1200	2400	4600	1700	3900	9400	2300	009	006	1800	1700	00/	1900	3600	3800
		CANAL	No.	L-1	L-1 Total-3	M-4	M-4	M-4	M-4	M-4	M-4	1 0 1	n = 1		Total-4	M .	M N C N	M = M	M-5	M-5	M N-N	Μ × 10 μ	∑ - <u>-</u>	L-2 +	1-2	1-3	n ∈ i	L-3	L-3	L-3	4-1-4	0 Kg	L-3	L-6	L-7	L-7	L-7	L-7	L-7	/ [\ 1 I	6-1	L-10

ENGINEERING AND DESIGN DATA Area 9 - Macedonia - Bethera - Gough

Sheet 4 of 4	TOTAL	TIMATED	Dollars (13)									7192.00	337,988.00	
Sheet	10											Ċ.	33.4	
		CULVERTS & BRIDGES - NEW	Length & Size (12)	301 - 30"			1 1	1 1 0 7		1	I I			
		CULVERTS	Length & Size (11)	-	1	l		l I	Į Į	101				
110	REQUIRED	RT. OF WAY	Ft. (10)	88 6	0 00	0 0	0 00	0 0	0 0	0 0	D 00)		
- Deniera - Gougn		VAY 1G	Ac. (9)	ි දිර දෙන	- - - - - -	v 0 -i C		5 г	O . ↓) (0 0	11.0	471.4	
Ionia - Det		EXCAVATION	Cu. Yds. (8)	4588	1560 0518	0100 1010	7 104	0100	3108	7.40	1007	22,052	1,080,339	
- maccuonia	SIONS	AVERAGE DEP TH	Ft. (7)	Юп	ט מ	Э и	о и	ΩШ	Ωμ	Ω	ט וע)		
Area 7	EL DINEN		Ft. (6)	თ ი	n (0 (n (n (n (m (n c)		
	CHANNI	TOP WIDTH	Ft. (5)	13	LG -	D 6	LG LO	η ·	Ε T	Σ ^τ	L3	C T		
		DISCHARGE	C. f. s. (4)	12	C	D 7	4. 80	30	:O 1	· (m (1	D		
		WATERSHED	Ac. (3)	134	174	234	308	492	44	20	53 (A	000		
			Ft. (2)	3100	1100	1700	008	3200	2100	200	800	14,900	460, 900	
		CANAL	No.	W-0	W - 0	M-6	W :	M-6	L-1	L-1	I-2	L-2 Total-6	Area 9 Total	

ENGINEERING AND DESIGN DATA Area 10 - Childsbury - Cordesville - Witherbee

Sheet 1 of 5	TOTAL	ESTIMATED COST	Dollars (13)										11,652.00						6430.00												14,913.00						-					
		SRIDGES - NEW	tength & Size (12)	-			30' - 36"	1	i	30' - 36"	30' = 18"	ŀ	ļ		1	40' - 24"	!			-		1	1 8	TO. O.T. Br.	-	1			40' - 24"	-			!	1 5	·	15' U.T. Br.			30' U.T. Br.	1	1	!
		CULVERTS	Length & Size		1 1	!			-	1	1 1	1		40' - 24"		1	!				-			1 1	1	.1	-	1	-	1		1	-	†	1 1	1	-	-	1	-		<u> </u>
herbee	REOUTRED	RT. OF WAY	Ft. (10)	38	0 88	38	38	38	38	38	ж ж	38	38	38	38	38	æ œ	g &		38	38	38	38	4 4 4 4	38	38	8 0	0 00	38	38		38	38	90 00	0 00	9 4	46	52	57	57	20 0	200
Cordesville - Witherbee		RT. OF WAY	Ac. (9)	0°0	0.0	1.8	2.4	0.7	2.5	, U	1.0	4.0	1.0	0.6	1.7	1,2	· · · · · · · · · · · · · · · · · · ·	1.0	0.2	2 -3	2 .8	0, 0 10, 1	0,0	. ↑	1.0	6.2	2 6	0.4	8.0	1.2	23.3	1 .9	1,4	000	, to	0	0.2	0.8	6.7	4.0	00 c	O n
ry - Cordes		EXCAVATION	Cu. Yda. (8)	7252	1480	3700	4736	1480	5032	5032	296	740	1924 35,668	1184	3404	2368	1480	3256	18,500	4588	5624	5032	5032	2405	2664	5920	4440	9888	1628	2368	47,306	3848	2812	1184	0886 0806	2040	408	1928	15,846	834	00000	02880
Childsbury -	SHORS	AVERAGE DEP TH	Ft.	ro r	o ro	ω Ω	Ŋ	S	Ω.	ເດີເ	ט ונ	ω N	വ	5	23	ເດ	υи	വറ		22	Ŋ	ro r	Ωш	O 10	ν ro	13	ro n	ט וכ	. ro	ıΩ		22	ıO	n n	טוכ	ω (λ	ιÓ	á	ಬ	រ ល	C π	o .
10.	444	MIDTH	Ft. (6)	m (n m	ന	က	თ	ო	က (ന ന) m	m	8	m	თ (m 0	က က		8	n	т (n <	4 rc) m	თ	ന	o 00	n (n)	က		က	m	m e	o m	. C	C	00	10	10	12	N H
Area	CHAMR	WIDTE	Ft. (5)	133	13 F	n €	13	13	13	13	n r	13	13	13	13	13	T I	L L H		13	13	13	L3	4 t	13	13	H3	.3 L	13	13		13	13	13	n €	16	16	18	20	20	222	N N N N N N N N N N N N N N N N N N N
		DI SCHARGE	c. f. s. (#)	24		99	16	25	11	o (σ ο α) H	m	9	14	20	Σ Λ Σ π	4 0 0		25	44	୍ଦ ପ	87.	103	2	11	13	0 46	9	10		14	88	40	 . €	131	131	164	197	197	888	DD NV NV NV
		WATERSHED	Ac. (3)	300	087.	1006	184	314	120	94	88	114	24	48	160	240	324	96		326	616	988	1208	1768	02	120	144	4.58	58	100		164	436	556	080	2328	2332	3000	3704	3710	4392	4520
		LENGTR	Ft. (2)	4900	1000	2500	3200	1000	3400	3400	1900	2000	1300	800	2300	1600	1000	2200	12,500	3100	3800	3400	3400	1300	1800	4000	3000	9008	1100	1600	31,600	2600	1900	800	0000	1000	200	800	5700	300	2200	000000000000000000000000000000000000000
		CAKAL	Ko.	M-1	7 F 1 X X	, M	L-1	L-1	L-2	L-3	L - 3	1 1 1	L-5 Total-1	M-2	M-2	% - ₩	N 0	L A	Total-2	M-3	M-3	M-3	ກ (Σ	n 0	L-1	1-2	1-3	4-17	L-5	L-5	Total-3	M-4	M-4	M-4	Z Z	M – M	M-4	M-4	M-4	M-A	M ;	M-4

DATA	Witherbee
AND DESIGN DATA	lle - \
AND	1ry · Cordesvi
ENGINEERING	- Childsbu
ENGIN	Area 10

Sheet 2 of 5	TO TAL	ESTIMATED COST	Dollars (13)																		,		-						46,541,00						7274.00							6344.00			
S		CULVERTS & BRIDGES - NEW	° -	1	ŀ	1	30' - 42"	-		15' U.T. Br.	!	30' - 48"	i	-1	30' - 48"	8	IS' U.T. Br.	30' - 24"	i	-	30' - 30"		30' - 42"	i	30' - 24"	30' - 36"	1			30' - 42"	-		30, - 40,	ŀ		40' - 24"		-					-	40' - 24"	-
			Length & Size (11)	-		!		!	-	1			-	1	1	-			-	ļ	-	-		-	1	-	1				!	40' - 72"	! !			1	40' - 48"	1		40' - 24"	i		-	!	1
wither Dec	REQUIRED	RT. OF WAY WIDTH	Ft. (10)	38	38	38	38	38	38	41	41	99 89	38	38	38	8 0	m m	98	38	38	38	38	38	38	38	8 0	200	0 80		38	38	38	D C	0 00		38	38	8 0	m e	0 8	38	3	38	38	38
COLUCSVIIIC - WIL		W A Y	Ac. (9)	6.0	1.3	1.8	8.6	ທ.0	2.0	3.7	0.10	. o.	8.0	. 53	2.5	2 0	0 0	. 0	0.0	2.2	1.0	0.8	2.1	0.4	0.7	H .	· · ·) (V	63.3	0.8	2.1	œ (0.0	, 50 H .	8.8	1.2	0° 0	n (. c.	7.1	, to	9.1	0.7	0.4	1.4
r) coraco		EXCAVATION	Cu. Yds. (8)	1776	2664	3552	5180	1036	3996	7682	2171	7844	1628	3108	5032	444	4444 888	1184	1036	4440	1924	1628	4144	740	1480	2220	1778	5032	133,535	1628	4292	1628	3996	4292	17,760	2368	5180	2516	888	1924	2960	18,352	1480	888	2812
Ominana i	SIONS	AVERAGE DEP TH	Ft. (7)	5	2	N	ر د	ıO.	ເດ ເ	ທ ເ	ດທ	ດທ	ιΩ	2	2	ro n	ט זכ) 10	2	22	ro	10	ທ	ಬ	ıo :	n n	οщ	. ro		ro	2	ıo ı	ט זינ	2		5	លេ	Ω N	ດທ	ນທ	10)	5	D.	ທ
	DIMEN	Σ	Ft. (6)	က	m	က	თ (m	თ •	4.	4) ຕ	ო	m	ო	თ (n m) m	က	က	ო	ო	ო	m	n	ന	n (າ ຕ		m	ო	m (n a) m		က	m (m (m (1	o m) m)	m	က	က
TATE OF THE PERSON NAMED IN COLUMN 1	CHANNEL	WIDTH	Ft. (5)	13	13	13	13	133	13	14	14 13	13	13	13	13	13	13	13	13	13	13	13	13	13	13	13	T C	13		13	13	13	L L	13		13	13	Σ T	L3	13	13)	13	13	13
		DISCHARGE	C. f. s. (4)	11	10	0	21	22	48	825	94	27	28	15	27	200	C 4	ည်း	7	21	6	11	14	14	ω	16	# 00	17		13	27	49	1.5	22		2	ر ا	. (- α	+	1 03	1	П	83	23
		WATERSHED	Ac. (3)	112	108	96	256	268	692	1320	1352	340	374	180	340	380	040 650	44	64	260	92	112	160	168	92	188	ь 100 г 100 г	206		144	348	696	140	268		88	304	444	2000	70.00	102		09	94	120
		LEN GTH	Ft. (2)	1200	1800	2400	3500	200	2700	4600	3700	5300	1100	2100	3400	300	300	800	700	3000	1300	1100	2800	200	1000	1500	1,500	3400	77,600	1100	2900	1100	2700	2900	12,000	1600	3500	1,700	1700	1300	2000	12,400	1000	009	1900
		CANAL	Ko.	L-1	L-2	L-3	L-4	L-4	L-4	L-4	L-4	L-6	L-6	L-7	L-7	L-7	L-7 T:-7	1-8	L-8	L-8	L-9	6-7	L-10	L-10	L-11	L-11	7 - 7	L-12	Total-4	M-5	M-5	ν 1Ω 1ι	M=0	L-1	Total-5	M-6	Θ (Σ :	9 (U	M - 6	1 1	2-1	Total-6	M-7	M-7	M-7

ENGINEERING AND DESIGN DATA Area 10 - Childsbury - Cordesville - Witherbee

Sheet 3 of 5 TOTAL 15,691,00 4671.00 14,024.00 4235.00 2734.00 Dollars (13) COST BRIDGES - NEW Lewoth & Size Br. Br CULVERTS & - 36" 36" - 36" - 48" 36" - 36" - 48" 30 " - 36" C. T. C.T. (12) 1 1 1 1 1 1 1 1 40, 404 30 1 40, 301 301 30 40, 30 301 30 4 15, 15: Length & Size 60" - 36" CULVERTS LOWERING (11) 1 1 1 1 1 1 | | | | | | 1 1 1 1 401 30 4 REQUIRED RT. OF WAY WIDTH Ft. 38 38 38 38 8 8 8 8 8 OF WAY CL EARING 0.1 2.3.2.4.2.0.7.0 4 °6 3 °1 8 °1 (6) 0.00 0.4 AC. EXCAVATION Cu. Yds. 4440 4884 4440 6216 1332 2516 6512 2812 3700 4144 2516 1778 1628 740 8140 1184 1480 2664 3848 3256 2220 2220 5624 9176 2220 2960 5032 40,848 1480 2960 2368 3700 2368 2516 1924 3404 41,144 14,208 888 (8) AVERAGE Ft. DEP TR CHAN'NEL DIMENSIORS വവവ വവവ வ்வவவ் 4 4 4 4 4 4 4 4 BOTTOR WIDTH (9) 0 0 0 n n n 00000 0000000000 WIDTH Ft. (5) 707 13 13 13 13 13 DISCHARGE c. f. s. (4) തതത a a m 7 111 112 115 21 21 21 22 2 00 9 00 00 WATERSHED 270 628 1204 1304 804 912 1102 140 284 128 402 702 792 1030 1576 1186 1186 116 340 520 134 184 232 166 340 480 168 600 288 152 300 420 92 128 238 82 82 86 270 Ac. (3) ENGTH 2200 1500 1500 3800 3000 3300 3300 3300 9600 1500 3400 4400 27,600 1700 1200 1100 600 4200 900 1700 800 1900 2500 2800 7200 27,800 5500 2000 1600 800 2500 900 1600 Ft: Total-10 Potal-11 Total-8 Total-7 Total-9 M-10 M-10 M-10 M-11 M-11 M-11 L-1 L-1 M-12 M-12 M-12 M-12 M-12 L-1 CANAL Z Z = 0 L-1 L-2 ¥0.

ENGINEERING AND DESIGN DATA Area 10 - Childsbury - Cordesville - Witherbee

Sheet 4 of 5	TOTAL ESTIMATED COST	Dollars (13)		8053.00																																	-						
	CULVERTS & BRIDGES - NEW	Siz	301 - 30"		30' - 54"		15' R.C. Br.		1	-	1	1	1			-	,	15' C.T. Br.	i	30 48.	! !		15' R.C. Br.				ı i		ł	30' - 42"	17.7 C U U	·		1	30' - 42"	60' - 24"	-			30' - 42"		1	
	CULVERTS	Length & Size (11)			-	1	-			1	-	1	1	1				-	1	1 1	1 1	-			-		!	1	1	1 1	1 1	-	1	1	-		-	I 1	1	1	-	1	
W MEMOL DOC	REQUIRED RT. OF WAY WIDTH	Ft. (10)	38		38	38	38	30 38	0 00	38	38	41	41	1 4 7 1 00	64	46	38	38	38	D C	m cc	38	38	38	38	m c	0 88	38	38	38	3 33	98	38	38	38		œ ç	n a	0 00	38	38	38	
	RT. OF WAY	Ac. (8)	0.4	10.4	4.1	1.2	o · o	7 . 7	T . T	. H	1.8	0.5	O (n w	9 8	0.8	°3	3.8	0.0	0 4 4	D. 4	5.7	2.1	5.4	8.4	מ מ	. ∪ . ∪	1.2	1.8	0.0	O W) 	4.4	2.2	1.8		I. 7	- α - α	2 0	1.8	1.4	0.6	
CHILDRAIN COLUCTION	EXCAVATION	Cu. Yds. (8)	888	21,016	8288	2368	1776	5476	1480	2664	3700	1002	1336	1428	18.768	4488	4588	7696	1184	2007	1974 8880	11,396	4292	10,804	9620	7,104	2516	2516	3700	1184	3996	2220	8880	4440	3700		3404	4696	4884	3552	2812	1184	
THE PERSON A	DIMENSIONS TTOM AVERAGE DTH DEPTH	Ft. (7)	വവ		Ŋ	ıΩ	N I	Ωμ	טו כ	Ω.	N	10 1	Ω 1	ט וכ	Ω (22	2	2	IO I	Ωш	ט זכ	n n	Ω	10	ıΩ I	ОК	0 10	N	20	2	n n) 10	Ŋ	Ω	22		n u	ט גר) IC	Ω (10	K)	
1	3 8 ≥	Ft. (6)	നന		m	m	n (n c	ാത	n (n)	ന	-4-	4.	4 C	00 (0	9	n	ന	m (n c	n m	m	က	n	m (n 0	n m	m	00	ന	m r) m	n	m	m		m (ൗ ന) (T	n m	m	က	
-	TOP WIDTH	Ft. (5)	13		13	13	13	T T) C	13	13	14	14	1 1	16	16	13	13	133	D C	7 F	13	13	13	13	F 6	H H	13	13	13	T T	13	13	13	13		m c	- H	- H	13	13	13	
	DISCHARGE	c. f. s. (4)	ന ന		9	7	11	0 H	0 0 0 0	3 6	61	ر ا	67	\ 0 0	00 00	101	4	10	10	ОП	ი თ	10	22	32	o (T 3	· 00	CV	m	ന	w m) 4	35	38	4	1	ω ₄	7 1.	- m	7	9	10	
	WATERSHED	Ac. (3)	154 182		358	426	760	1144	100 A C C C C C C C C C C C C C C C C C C	2906	5554	6074	6242	0 00 00 00 00 00 00 00 00 00 00 00 00 0	9312	10,130	244	662	678	2000	310 548	664	1956	2564	568	W 42 W	480	76	144	168	320	988	1752	2356	184		308	4 18	- H	200	338	୯୦୧	
	L EN GTH	Ft. (2)	600	14,200	5600	1600	1200	3700	1000	1800	2500	600	800	400	9200	2200	3100	5200	800	2400	1300 8000	7700	2900	7300	6500	4800	1700	1700	2500	800	3400	1500	0009	3000	2500		2300	2 CO CO CO	0000	2400	1900	800	
	CANAL	No.	1-2	Total-12	M-13	M-13	M-13	M 1 1 2 2	M M L L L	M-13	M-13	M-13	M-13	⊠	Σ X	M-13	L-1	1-2	1-2	F - C	5-1	L-5	L-5	L-5	1-6	1 0	1-7	1-8	L-8	0-J	の o	6-1	6-1	1-9	L-10		L-10	1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	L-14	L-14	L-14	

- AND DESIGN DALA
Spiry - Cordesville - Witherbee

Sheet 5 0: 5	ESTINATED COST	Dollars (13)	73,753,00	00.000		4277.00				7931.00	228,523.00
	CULVERTS & BRIDGES - NEW	Length & Size (12)			30, - 60,	I I	and the state of t	!	 40' - 18"		
		9 2			4	1	40, - 60,"		!!!	}	
AA BEERK B SACK	REQUIRED RT. OF WAY WIDTH	Ft. (10)	38		8 8 8 6	D O	38	38	38	388	
	RT. OF WAY	Ac. (9)	3.1		9.7	5.7	. ∪	0.2	0.0	11.9	321.1
Superior Sup	35	Cu. Yds. (8)	1480 6216 215.502	2001	9176	11,396	2664	444	12,284	2960	657,997
0	AVERAGE DEP TR	Ft.	иn		n n)	22 22	rç	n n	ro.	
	WIDTH WIDTH		ოო		m r	o	m m	n	ന ന	n	
	FOT	Ft. (5)	13		13	7	13	13	13	13	
	DISCLARGE	C. ř. s. (4)	Ηß	And the second s	r 0	0	r 0	C	19	n	
	A TERMINA	Ac. (3)	58 275	Control of Control of the Land	400	2/.4	140	340	1374	130	
		Ft. (2)	1000 4200 140.800	000 to#H	08800	7700	1800	300	8300	2000	426,800
	CANAL	No.	L-15 L-16	D T L T B D T L	M-14	M-14 Total-14	M-15	M-15	M-15	L-1 Total-15	Area 10 Grand Total

ENGINEERING AND DESIGN DATA Area 11 Jamestown Shulerville Honey Hill

REQUIRED OF WAY RT. OF WAY	EXCAVATION RT. OF WAY	BOTTOM AVERAGE EXCAVATION RT. OF WAY	BOTTOM AVERAGE EXCAVATION RT. OF WAY
EARING Ac. (9)	Cu. Yds. (8)		DEP TH Ft. Cu. Yds. (7) (8)
2.9	2664 1.3	5 5 2664	3 5 5 2984 2
0.0		5 15,665	8 8 5 15,665
1.2	2516 1.		3 3 5 2516 3 3 5 3404
4.0	3468	3458	3468
16.6		35,413	35,413
1	18,972	5 18	6 5 18
	9842	ι Ω	ο (
	15,290		000
	10,203	Ω Ω	Ω Ω
	13,776	ιΩ	25 5
	12,876		D
	9176	ഗ	വ
	14,615		D D
	11,766	សាធ	សាធ
	15,840		14
	14,080	5	14 5
	14,863	ıc	4
	5624	Юп	o с
	4588		O1 C
	5920	23	ю го
5.1	10,212	2	3
	10 ABO	Ľ	Ľ
	13,344		0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0
	36,738	ιΩ	26 5
	22,940	IO I	35
	12,964	Ω Ω	45
	18,520	12	45 5
	30,558	ις	45 5
	12,040	ı کا	60
	30,100	ו מו	000
12.7	36,120		и и
25.2	72,228	Ω	70 5
	22,224	ro.	70 5
	132,216	5	80 5
63.0	184,695	5 184	5 184
	5511	ט נט) 4) ro
	30,580		- 22

ATA	Hill
	ney
Z	. Ho
DESI	erville
DZZ	· Shul
	- Jamestown
-	
(17)	
Z	Area

Sheet 2 of 3	TOTAL	ESTIMATED COST	Dollars (13)																																		-							350,063.00			12,723,00
		CULVERTS &	Length & Size (12)	15' C.T. Br.	-				30' - 36"		1	-	15' U.T. Br.	(IS' C.T. Br.	-	İ	30 54"			1 C	;				!	30' - 42"	40' - 54"	15' C.T. Br.	40' - 36"	f	15' R.C. Br.		15' C.T. Br.	15' C.T. Br.	30' - 60"		30' - 48"	1 9	SOF R.C. Br.		1	30' - 54"		30' - 42"	i.	30' - 48"
		CULVERTS	Length & Size (11)		-		!	1		1		-		-		1		!		1		!		1	-	-	-		-	1		!	!		1	}	1	1	1		1	-	-				-
	REQUIRED	RT. OF WAY	Ft. (10)	46	46	49	44	55	38	38		38	38	38	24.	Q 1	7.0	200	1 4 0	0 0	0 0	0	38	38	38	55	38	38	49	38	(33	38	38	38	38	38	38	38	D 0	0 00	38	38		38	38	38
		WAY	Ac. (9)	5.8	1.7	10.9	7.8	13.7	4.1	5.1		25.2	2.0	Q &	. ° T	N 0	ń п Ö o	0 -	-1 0. 1. 0	o «	s rc	0	o. e	1.9	2.6	3.4	8.0	3.7	8.1	4.2	1	n 0	ъ. 6.	3, 21	4.6	2.6	7.9	က်ပ	က် က	ე ი և	0 0) m	8.9	458,6	6.0	7.3	ത സ
		波	Cu. Yda. (8)	12,852	3672	24,420	16,465	31,598	8288	10,360		5476	7400	3996	67.74	2004	11 809	2006 2006	3006	7 200	10 858		7844	3848	5328	0444	6364	7400	18,204	8436	()	TT, 040	10,656	7104	9176	5180	15,836	7104	10,656	11,390 0050	9668	4588	5920	1,161,441	12,136	14,800	11,840
date con m	SHOIS	AVERAGE DEP TH	Ft. (7)	ro.	N	N	N	N	10	ໝ		ıo ı	ΩI	Ωu	ת כ	Оп	ט ע) II	ס נר) IO) LC)	Ŋ	10	22	ಬ	N	Ŋ	2	ıΩ	U	י	ß	ιΩ	ſΩ	ın	ı م	ιΌ I	က် ယ	ο rc) IC	Ω (ıδ		ا با	மி	ń
20 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	- 1	Z -	Ft. (6)	C.	9	7	10	0	m	ന		თ (m (n t	- 0	. () (r	0 <	לי כי	o m) (r))	m	m	n	0	ო	n	7	က	c	n	m	က	m	ന	თ (ന (n 0	n (r	o m	ന	က	***************************************	6	m (
	CHAMN	TOP	Ft. (5)	16	16	17	12	19	13	13		E (n (L G	\ T	· - C	2 0	7 F	# CT	o en H ←	H ←)	13	13	13	19	13	13	17	13	C	O	13	13	13	13	13	13	13 T	T T	n en	13	13		13	T3	Σ
		DISCHARGE	c. f. s. (4)	70	73	78	59	66	22	59		S (S)	a c	4, 0	101	7 22	100	000	99) (X) IC)	37	49	59	136	16	35	78	83	(0	59	35	93	00	22	ر س ا	n 12	3.4	40	9	7		14	m (m	10
		WATERSHED	Ac. (3)	1080	1144	1248	880	1620	268	384		424	732	887.	2000	2 0 0 0	3602	300	548	806	720)	508	716	876	2424	188	476	1252	296	0	280	880	480	968	420	1692	308	1048	00 1 4 4	3344	336	428		988	2629	632
		LENGTH	Ft. (2)	6300	1800	11,000	8900	12,200	2600	7000		3700	2000	7,000	1,000	1200	2000	1900	1900	3500	2000)	5300	2600	3600	3000	4300	2000	8200	2400	0	0	7200	4800	6200	3500	10,700	4800	0027	4700	2700	3100	4000	296,800	8200	10,000	8000
		CANAL	Ko.	1-3	L-3	L-4	L-5	L-6	L-7	L-8		F-0	D (C	n c - 1 - 1		D C	- I		1,11	1.19	1 F	2	L-14	L-14	L-14	10-14	1-15	L-18	L-16	L-17	0	0	L-19	L-20	1-20	1-21	L-21	1-22	77-7	1.199	1-22	1-23	L-24	Total-3	M-4	M-4	L-1 Total-4

ENGINEERING AND DESIGN DATA
Area 11 - Jamestown - Shuldervillo Hanne Hann

Honey Hill	•	
Shulerville		
Jamestown -		
0		
=		
_		
Area		

Sheet 3 of 3	TOTAL	EST! MATED COST	Dollars (13)			5350.00			7594.00								15,730.00							-				25,391.00		5198.00		4469.00	00 0180	7. T			
		CULVERTS & BRIDGES - NEW	° (100	1	40' - 54"	15' U.T. Br.			-	i	30' - 60"	1 - C - 7 7 7 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	• •	1		30' C.T. Br.	1	1	-	1		[15' C.T. Br.	1	1		15' U.T. Br.	1	30' - 36"	i	15' C.T. Br.				
		~	Length & Size (II)					-			!		-		!				-		!		1 1	1		1	-			1		1					
2	REQUIRED	RT. OF WAY WIDTH	Ft. (10)	38	D 07		38	38		38	38	38	88 6	m a	000000000000000000000000000000000000000	38		38	38	88 8	m 0	0 0	0 00	38	38	38	38		38	38	38	38	38				
		RT. OF WAY	Ac. (9)	2.6	vs a 4. −	8 0 1	4.2	0.0	10.2	8.1	1.7	2.1	4 , Ç (J C	, H	0° 4	23.4	7.3	7.1	25.7	Σ α	⊢ ⊢ ئ ⊂	4 4 5 t	හ භූ	4 .4	1 .4	ත. ස	38.3	2.8	7.8	3.7	ದಿ ಬೆ	5.1	H 0.000	2		
		EXCAVATION	Cu. Yds. (8)	5180	4736	16,132	8436	12,136	20,572	16,280	3404	4144	9324	1480	2368	8140	47,212	14,800	14,208	5476	3700	5002	8288	6880	0888	2812	7844	77,404	5624	10,064 15,688	7400	3256	10,212	1,855,944	# # 0 0 0 0 1		
THE STATE OF THE S	SIONS	AVERAGE DEP TH	Ft. (7)	2	ດທ)	5	S		5	2	ر د	in i	ממ	വ	ro		5	ഹ	រ ប	ο r.	ט וכ	ວ ເວ	Ŋ	Ω	2	D		2	ro	ις	Ω	2				
	CHANNEL DIMENSIONS	BO TTO M WIDTH	Ft. (6)	0	നെ)	n	m		n	ന	ന	m (ന ന) m	m		n	ന	ကေ	n 0	n (r	o m	თ	က	က	က		m	ന	m	ო	m				
	CHANN	WIDTH	Ft. (5)	13	n ⊏) H	13	13		13	13	13	m 4	n e	13	13		13	13	13	L L	7 F	13	13	13	13	13		13	13	13	13	13				
		DISCHARGE	c. f. s. (4)	4	T (C)	0	15		12	13	27	ω «	N O	10	7		25	47	61	4 (T T	D 60	0	11	18	4		15	ಬಿತ	4	o o	10				
		WATERSHED	Ac. (3)	456	0996	0000	564	1072		816	872	2128	512	4 07 72	640	458		1976	4088	5688	0 0	1110	372	372	748	1252	440		1060	1780	444	528	624				
		32	Ft. (2)	3500	3200	10,900	5700	8200	13,900	11,000	2300	2800	6300	1000	1600	5500	31,900	10,000	0096	3700	2000	1400	5600	4500	0009	1900	5300	52,300	3800	6800	2000	2200	0000	0000	*		
		CANAL	No.	M-5	M-5	Total-5	M-6	M-8	Total-6	M-7	M-7	M-7	Ľ-1	1 [-12	2 2 □	L-3	Total-7	M-8	M-8	M-8	L-1	1 -	1 2	L-3	L-4	L-4		Total-8	M-9	M-9 Total-9	M-10	M-10 Total-10	M-11				

Area 12 - Bethera - Huger - Eccles Church - Green Bay

ESTIMATED Sheet 1 of Dollars TO TAL (=3) COST CULVERTS & BRIDGES - NEW Length & Size Br. Br. Br. Br. BH Br. C.T. Br. 48" 48" 54" 24" 30 " - 54" 48" 48." C.T. U.T. R. C. R. C. C.T. C . T . (12) ı 30 4 409 40,1 308 404 40' 401 404 30.5 15' 15' 30.1 45 454 30 60 4 Length & Size CULVERTS LOWERING REQUIRED RT. OF WAY WIDTH Ft. 38 38 38 38 38 44 44 44 55 55 68 68 38 OF WAY CLEARING Ac. (9) 3.7 1.1 2.3 EXCAVATION Cu. Yde. 13,425 2220 13,172 11,544 5500 6500 9666 11,814 5772 7548 3996 5772 5328 4736 3700 1776 10,656 7400 7548 8140 6660 5100 6734 5632 9856 9916 5177 10,608 2363 5624 18,944 4676 15,906 15,096 11,100 12,232 15,036 15,096 11,544 18,648 18,795 25,256 (8) AVERAGE Ft. DEP TH CHANNEL DINENSIONS BO TTOM WIDTH 0 آئي خلا (9) HLCIM T0P (8) 13 13 113 114 115 116 118 119 119 119 119 113 DISCHARGE c. f. s. (4) 238 258 9 12 32 52 57 57 63 82 92 101 101 139 145 12 254 WATERSHED 7796 8268 648 852 1612 316 3424 5376 6528 228 884 2564 4628 5096 5784 7904 9528 10,400 14,788 15,484 26,780 28,416 28,920 29,556 468 236 220 524 688 296 448 368 824 10,120 800 11,176 27,552 28,160 30,432 30,872 Ac. (3) LENGTH 2200 6800 4400 1300 1800 2500 3500 2800 4400 3900 5100 2700 3900 1600 3800 3200 10,200 1200 5000 2,800 1500 8900 5500 2800 3600 6600 7200 2600 1600 2800 3600 10,500 Ft. L-11 L-11 L-11 L-11 L-11 L-12 L-11 L-11 CAHAL L - 11L - 11L-2 M-1 $\mathbb{M}_{-}\mathbb{1}$ M-1 $\mathbb{M}-\mathbb{1}$ M-1 \mathbb{M}_{-1} $\mathbb{M}-\mathbb{1}$ L-1

Area 12 - Bethera - Huger - Eccles Church - Green Bay

Sheet 2 of 5	TOTAL	ESTIMATED COST	Dollars (13)																												208,255.00																	
		CULVERTS & BRIDGES - NEW	\$ S12 2)	-	-		30' - 42"		-	1 1	30' - 24"	50' - 24"	!	1 (15' C.T. Br.		07 1 00	30' = 48"		-	-		15' U.T. Br.	i	30' - 42"	1		!	-	7-		1	1	30' C.T. Br.	J. O.	• •		!	45' C.T. Br.		1 6	IS' C.T. Br.	E	LO C.T. Br.			!	ļ
'n		CULVERTS	Length & Size (11)			i	1		1	-	1			1	-		1 1			!	1	1		!		1		-	-	To see		-	-	1	1		-	-	1	1	1		-	1		-	!	
- Green Bay	REQUIRED	RT. OF WAY	Ft. (10)	38	38	38	38	38	38	38	38	,	38	38	an t	4 C	0 0	0 m		38	38	38		38	38	38	38	38	38	38		38	38	41	4 L	υ Ω	55	62	62	62	68	38	80 0	B 0	m c	38	38	38
Church		RT. OF WAY	Ac. (9)		5.5	3.6	œ. ش	5.5	4.9	1.2	0.5		4.6	w . ∞ .	4 t	· · · · · · · · · · · · · · · · · · ·	· 4	, a		1.3	2.4	4.2		4.1	4.0	2 .6	о • Э	3 .7	8.50	1.5	285.2	4.8	4 3	0.7	n n Öa	1	2.5	8 9	6.0	8,	80 .	4.1	⊔ п О° (v 4 vi		1 0	· · ·	5.7
- Betnera - Huger - Eccles		EXCAVATION	Cu. Yds. (8)	7104	11,248	7252	7696	11,100	9916	2368	5180		9176	7696	9324	3307	D176	17,168		2664	4884	8436		8288	7992	5180	10,656	7548	5624	2960	616,396	9620	9028	1670	14,000	3856	5698	8820	17,010	11,340	7040	8288	3848	10,656	2 2 2 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7400	2518	11,544
ra - rug	SIONS	AVERAGE DEP TH	Ft. (7)	22	2	2	2	2	Ŋ	D	2		ا ما	<u>ه</u> ۱	n 4	ОМ	ט גמ) 10		Ŋ	ro	2		ω	ಬ	υ	n	5	υ	D.		N	വ	IO II	ט גר) 10	10	2	2	Ω	10 I	۱ ۵	C II	ΩL	O 10) LC) 1C	22
	2	BOTTOM		m	m	m	m	ო	m	m	ო		m (m (;n ←	4 0	ე თ) m		m	m	e		m	ო	ന	ന	m	ო	m		m	m	4 1	ο α	0 00	0	12	12	12	14	m (m c	m c	n r) (r.) m	, m
Area 12	Z.	TOP WIDTH	Ft. (5)	13	13	13	13	13	13	13	13		ε ·	n .	m -	4 6) K	0 C		13	13	13		13	13	13	13	13	13	13		13	13	44	. α	18	19	22	22	22	24	ກ ເ ⊢	- H	T C	L L	o (c)	H H	13
W		DISCHARGE	C. f. s. (4)	10	19	60	Φ	23	12	0	4	,	TT	D 1000	, m	000	· L	- 13		17	9	0		11	Φ	4	00	ιΩ	10	ന		7	231	φ n	\ o	0 0	66	120	127	130	140	0 !	23 62	1 5	თ თ ⊣ თ		4 63	13
		WATERSHED	Ac. (3)	644	1412	504	508	1740	780	352	200		696	2192	3048	44.1.0	404	1020		1100	360	572		728	496	236	520	272	656	172		388	1544	4252	0140	0000	10,232	12,372	13,328	13,648	14,952	656	976	008	9636	2007	1660	006
		LENGTH	Ft. (2)	4800	0094	4900	5200	7500	6700	1600	3500		6200	5200	6300	N TOO	0000	11.600		1800	3300	2700		5600	2400	3500	7200	5100	3800	2000	329,700	6500	6100	1000	7,500	1600	2200	2800	5400	3600	3000	5600	2600	7,200	1900	2000	1700	7800
		CANAL	.0N	1-13	1-14	L-15	L-16	L-17	L-18	L-19	1-20		L-21	L-21	1-21	L-21	1.03	1 2 2 4	1	L-24	1-25	L-25		1-26	L-27	L-28	T-29	L-30	L-30	L-31	Total-1	M-2	M-2	M :	Z Z	S CV	M-2	M-2	M-2	M-2	₩ 1 W	[-1	- F	N C .	0 0 0] [] (11

Area 12 - Bethera - Huger - Eccles Church - Green Bay

Sheet 3 of 5	TOTAL	ESTIMATED COST	Dollars (13)																					1						107,121,00																	
		CULVERTS & BRIDGES - NEW	Length & Size (12)		1	30' - 48"				15' C.T. Br.	-	-		LO C. E. DE.	100	C. J			-	30' - 60"	-	1		-	1	-		1	1		40' - 54"	l	30' C.T. Br.				1		1	1		i	1	30 - 30"	-	15' C.T. Br.	
Bay		CULVERTS	Length & Size (11)						l		ļ	-					1		-	1	-	-	-	-		1		1				!	}	-	!	-		1 1	1	-	-	!	-	-	1		
Green	REQUIRED	RT. OF WAY	Ft. (10)	38	38	38	38	38	38	38	38	338	0 0 0	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0 00		38	38	38	38	38	38	38	38		38	38	338	D 0	D N	38	38	38	41	44	/.0	0 00	23	2,4	38	38	38	38	38	38	38	38
Church -		RT. OF WAY	Ac. (9)	10 .2	1.3	3.4	4.6	0.7	3.0	4.3	0, 0	Q (1 F	, c)	2.2	6, 4	2.1	1.7	3 °0	2.7	2.1	8.0	adequate	4 •6	2.1	ເນ ≁ ເນ໋ ເ	7 1	155.2	2.6	rv rů	5,4	₩.2	ω,	ω c	0.2	- 4 H (C)	0 0	က္	2. 5.	4.2	6.7	2.7	4 .3		1 2 2 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5
ger - Eccles		EXCAVATION	Cu. Yds. (8)	20,572	2664	8089	9176	1480	6068	8732	10,064	3888	37,00 17,836	3400	8000		4440	9916	4292	3404	6068	5478	4292		s considered	9.176	4144	6660	SOLO	324,877	5328	11,100	10,952	4342	1665	OTO OTO	17 600	10,892	25, 134	7696	4884	8436	15,836	5476	8584	4736	4440 3108
ra - Huger	DIMENSIONS	AVERAGE DEP TH	ft.	20	N	Ŋ	10	വ	N	10	ו מו	υn	ט ע) IC.	10)	10	10	10	10	IJ	10	10	10	tru	N	10 1	Ωи	ЭШ	೧	22	10	20	10 :	ת ט	Эи	ט וכ	ט נט	10	D.	Ю	10	מ	Ω	10	n :	מ יט'
- Bethera		BOTTOM	Ft. (6)	က	n	n	n	n	ന	ന	ന	n	יז מי	o (*.) (T)	ന	က	က	n	n	n	ന	က	as cons	ന	ന	י מי	0 (ກ	8	က	က	4	v () F	# T	16	18	ന	က	က	က	n	n	က	m m
Area 12	CHANNEL	TOPWIDTH	Ft.	13	13	13	13	133	73	± 13	13	n €	n F	O €.) (*;	i	13	13	13	13	13	13	13			13	13	ກ ເ ∃ ເ	۲ ا د	r T	13	13	13	14	12	0 0	1 0	28	28	13	13	13	13	13	13	13	13
A		DISCHARGE	c. f. s. (v)	17	20	IJ	7	15	21	56	10	4 4	t. €	1 + 5			18	10	n	7	11	4	က	សព	Present	ıΩ	17	, ,	ч ч	0	4	17	24	45	57	1 T T T	15. 15.	163	186	7	22	25	13	4	IJ	13	14 5
		WATERSHED	Ac. (3)	1224	1508	248	432	1076	1572	2064	616	240	166 878	0.40	400)	1284	644	128	424	740	188	124	252	596	316	1232	456	367	202	444	1208	1848	3880	5140	14 380	18 404	17,932	20,916	412	1636	1956	968	240	308	822	268
Commenter of the control of the cont		LENGTH	Ft. (2)	13,900	1800	4600	6200	1000	4100	2900	0890	2700	700	H 000 100 100 100 100 100 100 100 100 10	8100	1	3000	0049	2900	2300	4100	3700	2900	4400		6200	0000	4500) (O) (O) (O) (O) (O) (O) (O) (O) (O) (O	3700	3600	7500	7400	2600	0006	1800	1 LOOO	8800	2900	5200	3300	2400	10,700	3700	2800	3200	3000
		CANAL	0 =	L-5	L-5	L-6	L-7	L-7	L-7	L-7	I-8	- L - G	1-10	1.1.1	1 1 1	2	L-12	L-13	L-14	L-14	L-14	L-15	L-16	L-17	L-18	L-19	L-19	L-20	1 1 2	Total-2	M-3	M-3	M-3	M-3	n (n ∈ ×) (°)	M-3	M-3	L-1	L-1	L-1	L-2	L-3	L-4	L-4	L-4

ENGINEERING AND DESIGN DATA Area 12 - Bethera - Huger - Eccles Church - Green Bay

Sheet 4 of 5	TOTAL ESTIMATED	COST Dollars (13)							•																						_												
	_S	BRIDGES - NEW Length & Size (12)		-		1		! !	-	15' C.T. Br.	Į į	1			1	i	50' - 36" 15' C.T. Br.	C.T.		15' C.T. Br.	1	1	-			-	30' - 36"	1	101			i	30' - 54"	Ì	15' C.T. Br.	i	30' - 24"	1	C. I.	15' C.T. Br.	1	1	30' + 48"
	CULVERTS	LOWERING Length & Size (11)		1	-		!	l }	î I	-	-		1		!			1	-	1	-	dua pan				-	-	!	I I	<u> </u>	1	-	1	-	1	!	-	1	-	1	!		†]
	REQUIRED RT. OF WAY	WIDTH Ft. (10)	38	38	44				38	38	46	C	000		41	38	38	38	38	38	38	38	88	38	99 88	38	38	38	000	0 80	38	38	38	38	38	38	38	38	41	46	46	22	38
CHARCH	>-	CLEARING Ac. (9)	4.3	4 .5	2.0		adequate			6.5		d adequate	4 Q	2	8.8	o.e	4.5	1.9	2 .7	1.9	1.5			0 C	N F	4.4	2.2	0 r	D & C	7.8	3.8	4.3	v Š	2.1	0.4	1.9	4.0	4.8	o•e	0	2.7	4°.5	ມ ຄ ຜູ້ດໍ
	EXCAVATION	Cu. Yds. (8)	8584	9028	4255		is considered	ט מ		13,024		is considere	10 000	200	5845	7844	9028	3848	5476	3848	2960	10,212	5920	1924	5032	8880	4440	1184	2040	15,688	6364	8732	10,508	4292	888	3848	8140	9620	8183	1836	5916	10,360	3256 7252
The same	AVERAGE	DEP TH F t. (7)	22	Ω	- τ 0 +			constructed	5 2	5	Ω.	stri	ט נכ)	10	10 1	Ω	2	2	Ω	ro.	n	IO I	0 10	OI C	2	N	ЮП	Э и	о го	Ω	ιΩ	ω	Ω	2	Ω	Ŋ	ιÇ	22	D.	ı oı	υı	വവ
- Demera		WIDTH Ft. (6)	က	က	Ω Ω	g U	n n	บ ถ	. m	m	0	ď	n 0)	4	m	m	က	က	က	က	m	ന	თ ი) ന) ෆ	က	m (0 0	n m	თ	က	ന	ო	က	ო	ന	ന	4	0	ω c	ת כ	നന
7 mark	TOP	WIDTH Ft. (5)	13	13			ent canal			13		Present canal) F	C C	14	13	13	13	13	13	13	13	13	T T) F	13	13	13	7 6	n m	13	13	13	13	13	13	13	13	14	16	19	D C	13
7	DISCHARGE	c. f. s. (4)	36	57	84	Present	Present	Present	30	58	107	Pres	7. 4. @	3	70	31	o	14	27	33	37	0	വ	m c	ന ന H	0 0	m	۲.	T ((18	22	7	©	10	13	14	က	49	99	92	109	137	25
	WATERSHED	Ac. (3)	452	852	1388	1484	3408	4028 5184	388	848	1804	1852	400	0	1096	408	544	992	2140	2652	3060	348	288	0 00 0	160	336	176	404	212	1288	1688	412	368	620	876	966	188	969	1004	1560	1836	2436	108
	LENGTH	Ft. (2)	5800	6100	2300				3600	8800	4000	0	12 000	12,300	3500	5300	6100	2600	3700	2600	2000	0069	4000	0 00	3400	0009	3000	800	2000	10,600	4300	2900	7100	2900	900	2600	5500	8500	4900	006	2900	4000	2200
	CANAL	. (_)	1-6	I-6	L-8	P-0	. L-6	1 L-6	7-1	97	L-8	1-8	1 10	D- ⊤	L-10	L-11	L-12	L-12	L-12	L-12	L-12	L-13	L-14	L-14	1.15	L-16	L-17	L-17	L-18	L-1-1-0	L-20	L-21	L-22	L-22	1-22	L-22	L-23	L-24	L-24	L-24	L-24	L-24	L-25

ENGINEERING AND DESIGN DATA Area 12 · Bethera · Huger · Eccles Church · Green Bay

Sheet 5 of 5	TOTAL	ESTIMATED	Dollars (13)		,				154,994.00				6260.00				5857.00							12 452.00									19,374,00	514,313,00		 	
		TS &	~~·	i	30' - 48"		1	1		15' C,T' Br.		1		30' - 42"	-	301 - 24"		40' - 42"		i			50' - 30"	50' - 36"	15' C.T. Br.	+ I	15' C.T. Br.	i	30' - 36"			30' - 54"					
a y		CULVERTS	Length & Siza (11)	1		-	1	1 1		-	-	-		1	-	1			-	1 1	1		i I				1	-	1	1	i !	1					
- Green Day	REQUIRED	RT. OF WAY	Ft. (10)	88	D a	0 0	m a	0 00)	38	38	38		38	0 0 0 0	0 00)	38	38	38	38	38	38	38	88	380	38	38	38	00 00	0 80	38					
CHUICH		RT. OF WAY	Ac. (9)	6.0	n 0	, v	⊣ o Ω α	5 ← 5 π;		2.0	0; 0;	4.3	₹°0	ω 	1.7	 	00 H	4.3	1.2	1.8	0 .4	5.7	1.6	0 5 7	5.0	4.0	2.4	1.7	o 0	n c	5 03 0.	4.3	27.8	726.8			
- Deliefa - Higer - Eccies		EXCAVATION	Cu. Yds. (8)	1776	4000	07.72	3700	0000	472,864	3996	5772	8584	18,352	7696	3404	C963	17,168	8584	2368	3700	740	11,396	3256	1036	10,084	8140		3404	5772	0286	5180	8584	55,944	1,536,681			
ra - rin	SIONS	AVERAGE DEP TH	Ft. (7)	10 1	O Li) L	O 10) rc)	5	ιΩ	ro		ro	N R	טוכ)	5	ιΩ	22	Ω	ιΩ	n	N	2	20	ıo	Ω	ທເ	O R	o ro	Ŋ					
· Deine	CHANNEL DIMENSIONS	MOTTOM WIDTH	Ft. (6)	000	n (r	0 0	ന ന	o e:)	e	(n)	(C)		m	ന ന	o (1))	co	n	en	ന	ന	ආ	m	m	ന	က	60	m (n 0	o (n	ec			·		
Area 12	CHANN	TOP	Ft. (5)	13	T T	C F	L3	F F)	13	13	13		13	C C	O 67.		13	13	13	13	13	13	H 3	13	13	13	13	13	LG	13	13					
		DISCHARGE	C. f. 8.	39	0 0	D 6	T 30	100)	15	49	28		22	41	j (-	1	0	۵	10	11	22	Q		0	16	21	34	ស រ	ο <u>κ</u>	H) 4	7					
		WATERSHED	A C. (3)	540	0 80	477	148	108)	180	708	368		280	572	.116)	336	496	624	744	1600	104	408	556	1124	1596	2776	256	4000	224	432					
A STATE OF THE STATE OF		LENGTR	Ft. (2)	1200	0008	3300	2200	0000	280,000	2700	3900	5800	12,400	5200	2300	0000	11,600	5800	1600	2500	200	0044	2200	2.1,000	6800	5500	3300	2300	3800	9700	3500	5800	37,800	887,800			
		CANAL	No.	L-26	1 20	D 520	L-29	1.130	Total-3	M-4	M-4		Total-4	M-5	M - X		Total-5	W-8	M – 6	9 W	M-6	M-6	L-1	Total La	M-7	M-7	M-7	M -7	L-1	1 5	L -3	L-4	Total-7	Area 12 Grand Total			

ENGINEERING AND DESIGN DATA

2969.00 5830.00 6946.00 ESTIMATED 4546.00 22,841.00 Sheet 1 of Dollars TOTAL CO ST (3) BRIDGES - NEW Length & Size Br. Br. Br Br CULVERTS & 48" 42" 30" 42" 36, 30" C.I. C.T. U.T. (13) 1 | | | 1 1 | Į 1 1 1 1 20 30 30 30 30 30 15, 15' 15' CULVERTS LOWERING Length & Size 72" - 72" Area 13 - Pompion Chapel - Charity Church - Wando - Daniel's Island 1 1 1 1 1 1 1 1 | | | 1 | | 1 40. 40 REQUIRED RT. OF WAY WIDTH Ft. 38 38 38 38 38 38 38 44 55 62 68 73 OF WAY RT. OF WAY adequate adequate adequate adequate 6.8 1.0 3.2 11.0 1.5 0.7 2.6 1.2 4.5 5.1 3.2 0.4 2 5 2.8 13.9 3.6 2.8 1.6 33.3 Ac. (9) considered considered considered EXCAVATION conside Cu. Yds. 2812 6512 34,020 8800 7002 5328 2516 740 5624 2368 15,540 3108 4440 6512 3404 2220 1924 1332 5920 2072 5624 1480 9324 10,360 5032 1480 6956 5920 4588 2072 1924 1332 3256 8089 16,280 8140 5624 66,896 13,616 22,200 13,468 8 13 S AVERAGE constructed constructed constructed D EP TH <u>.</u> 2 CHANNEL DIMENSIONS S വവവ 0 0 0 20 20 20 2 2 2 വവ 2 2 2 BO TTOM WIDTH (9) ന നനന നനന 0000 ന ന ന ന വ ത 12 14 16 16 23 as 33 canal canal canal canal TOP WIDTH 13 13 13 13 13 13 13 13 15. 22 22 24 25 25 13 13 Ft. Present 9 11 12 5 Present Present Present DISCHARGE c. f.s. (4) 14 111 28 4 10 00 m 22 30 31 37 0400400 13 76 25 190 WATERSHED 3276 28 620 1000 1204 2916 236 312 480 156 1232 2416 2456 3108 320 156 4244 114 356 68 308 528 738 814 1686 120 248 284 640 852 296 264 404 Ac. (3) 3100 1400 1300 1300 900 4000 900 2200 4600 9200 1400 4400 8400 2500 3600 2100 3000 1900 7000 4400 500 3800 3400 1000 4700 4000 5500 3800 3800 5200 10,800 1900 1700 6300 45,200 15,000 LEN GTH 11,000 500 Ft. Total-2 Total-3 Total-4 Total-5 Total-1 CANAL M-5 M-5 M M -6 M -6 M -6 . : : M-1 M-1 L-1M-3 M-3 M-3 1-1 M-6 1-1 M-6 M-6 M - 4

Area 13 - Pompion Chapel - Charity Church - Wando - Daniel's Island

Sheet 2 of 5	TOTAL ESTIMATED COST	Dollars (13)																		,													120,088.00							16,829.00	2848.00
	CULVERTS &	& Si 2)		1	!		1	50' - 36"		30' - 36"	ı		!	45' C.T. Br.		301 - 40"	i	1	1		15' U.T. Br.		30' - 30"		30' - 36"	1	30' - 30"		;	*				U.T.	15' R.C. Br.	ر ا د ا د					30' - 36"
	CULVERTS	z e			1		1	-		!	Í	!	1		!	1 [1	-		1	1			-	1	1	1	-	1			 		1			1	- 1	-		1
	REQUIRED RT. OF WAY WIDTH	Ft. (10)	78	137	T 20	D 00	38	38	38	38	0 00	44	46	52	j) (D 000	0 80	38	38	38	41	4 r.) (C)	38	38	38	D 00	38	38	38	80 O	0 80		38	41	44 0 R) LO	38	38		38
	RT. OF WAY	Ac. (9)	7.8	° ° °	4 · C	7 6	2.0	4.6	1.2	0, 7 © 17	4.0	0.0	1.8	4.00	18.7	o a	0.4	8.2	1.9	T .	0.4	ດ ທ່	, T	0 . U	83	2 .	H 4	o.e	5.2	3.7		, w , w	173.8	4 .7	où •	i∕) u	0 0	1.4	5.4	88.6	4.0
	-	Cu. Yds. (8)	20,022	9666	16, 304	\$310 9176	3996	9324	2368	5180	2072	6920	4080	7712	49,000	7696	8140	16,576	3848	2960	8350	19.684	3848	6956	4588	2368	3700	7844	10,508	7400	32206	7696	389,590	9472	4676	0100	6507	2812	10,952	47, 793	8140
O NO TO NUMBER	E E	Ft.	2	ID III	၁ ဖ	מו מ	Ω Ω	Ω	Ω	n u	מוכ	Ŋ	Ŋ	IO II	Ω μ	ט נמ	Ω (S	Ŋ	ın ı	n n	מו כו	ω ω	S	Ω	IO II	מו מ	Ω	D.	ו מו	א ט	Ω Ω		Ω	n n	οч	ט נט	20	Ŋ		N
1 0	BOTTOM			40		n m	n	m	က	ന ര	ാ ന	ſΩ	0	00 0	N C	ാന	n	හ	က	თ -	4 a	o 0	n	8	က	m (n a	n 00	හ	o) (n 0	o 00		e	4 4	D С	0 00	Ø	m		ო
2 24 20	TOP	Ft. (5)	28	0 1) t	D 6.	13	13	13	H -	T 1	15	16	18	, u	o co	13	13	13	13	14	G F	13	13	13	H 0	n ←	13	13	13	Ta Ta	130		13	14	ο C	18	13	13		13
	DISCHARGE	c. f. s. (4)	242	400	n c	. K.	14	37	54	O (1	57	80	88	134	aTO	200	13	42	15	თ (თ ≀	1 48	148	13	22	13	න ර	ט מ	19	25	22 1	D C ((27		39	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0.00	131	œ	37		4
	WATERSHED	Ac. (3)	4840	11,272	12,455	47.2	168	504	780	232	844	1276	1528	2372	2000	240	148	584	176	444	1128	2708	144	276	140	288	0000	224	328	272	9890	360		540	2000	1220 0148	22.98	84	508		216
manuformation of the state of t	LENGTH	Ft. (2)	4700	1200	0041	007.1	2700	6300	1600	3500	1400	3400	2000	3200	0086	2000	5500	11,200	2600	2000	2600	7,800	2600	4700	3100	1500	2000	5300	7100	5000	2200	5200	181,800	6400	2800	3400	2700	1900	7400	400	5500
	CANAL	No.	M-6	0 - Z	M-0	1 - 1	L-3	L-4	L-4	L-5		I-6	L-6	1-0 0-1	1	0 1 1	F-9	L-10	L-11	11-12	1-11		L-12	L-13	L-14	L-14	01-12 01-12	1-17	L-18	L-19	01-1	07-II	Total-8	M-7	Z X	2 3	N =	L-1	1-2	1	0-1:40E

ENGINEERING AND DESIGN DATA

ESTIMATED COST 2884.00 1991.00 5362.00 1892.00 2250.00 5247.00 18,078.00 2220.00 3304.00 3388.00 Sheet 3 of Dollars (13) Br. BRIDGES - NEW Length & Size CULVERTS & - 36" - 42" 42" 30" - 48' 30, 36 U.T. (12) 1 | 1 1 1 1 ı 1 30 40 4 30 30 30 30 4 30 1 15 CULVERTS LOWERING Length & Size - 42" Area 13 - Pompion Chapel - Charity Church - Wando - Daniel's Island $\widehat{\Xi}$ | | | | | 1 -401 REQUIRED RT. OF WAY WIDTH Ft. (10) 38 38 38 38 38 38 38 38 38 38 38 38 38 38 38 38 38 38 38 OF WAY CLEARING 8 8 8 8 8 0 0 0 0 0 2.6 3.7 1.8 0.8 1.8 1.9 6.6 1.0 0.5 0.0 0.0 1.8 4.1 3.0 3.7 3.7 4.8 4.8 1.5 1.8 1.7 1.5 4.8 (6) RT. EXCAVATION Cu. Yds. 9620 5624 5328 5180 7400 5328 5328 7548 7548 3552 3256 1628 3552 3848 13,320 1036 1184 740 7548 1036 1776 3700 3848 8288 7400_. 7400 9620 9620 9620 2960 3700 3404 3108 5624 17,908 5180 55,648 15,836 (8) AVERAGE D EP TH £ (7) Ŋ Ŋ 22222 CHANNEL DIMENSIONS Ŋ Ŋ S 22 22 Ŋ Ŋ 2 2 2 2 2 Ŋ BO TTOM WIDTH (9) <u>ئ</u> 4 ന n 3 00000 0000000000000 m m 000 n ന 00000 TOP WIDTH Ft. (5) 13 13 13 13 13 13 13 13 13 13 13 13 13 13 13 13 DISCHARGE c. f.s. (4) 9 Ŋ Ŋ 222992 Θ (n) n Ŋ 220022 WATERSHED 104 372 536 1644 1964 520 900 1892 2088 672 188 304 324 108 320 496 108 2332 132 360 304 244 524 170 208 332 Ac. (3) 800 500 5100 5000 6500 6500 2000 2300 2100 5800 4100 5000 6500 6500 2400 1100 2400 2600 9000 1300 700 1200 2600 5600 6500 6500 3800 3800 3600 3500 5000 3600 3600 5100 5100 700 2500 3500 37,600 10,700 LEN GTH 12,100 Ft. (2) M-17 Total-17 Total-15 Total-16 Total -18 Total-14 Total-11 Total-13 Total-10 Total-12 Total-9 M - 16M-18 M-19 M-19 M-19 M-19 M-19 M-15 M-15 M-15 M-15 M - 13M-10M-11 L-1 M-14 M-14 L-1 L-2 M - 12CANAL $\widehat{\Xi}$ M-9 % 0×

ENGINEERING AND DESIGN DATA

· Pompion Chapel - Charity Church - Wando - Daniel's Island

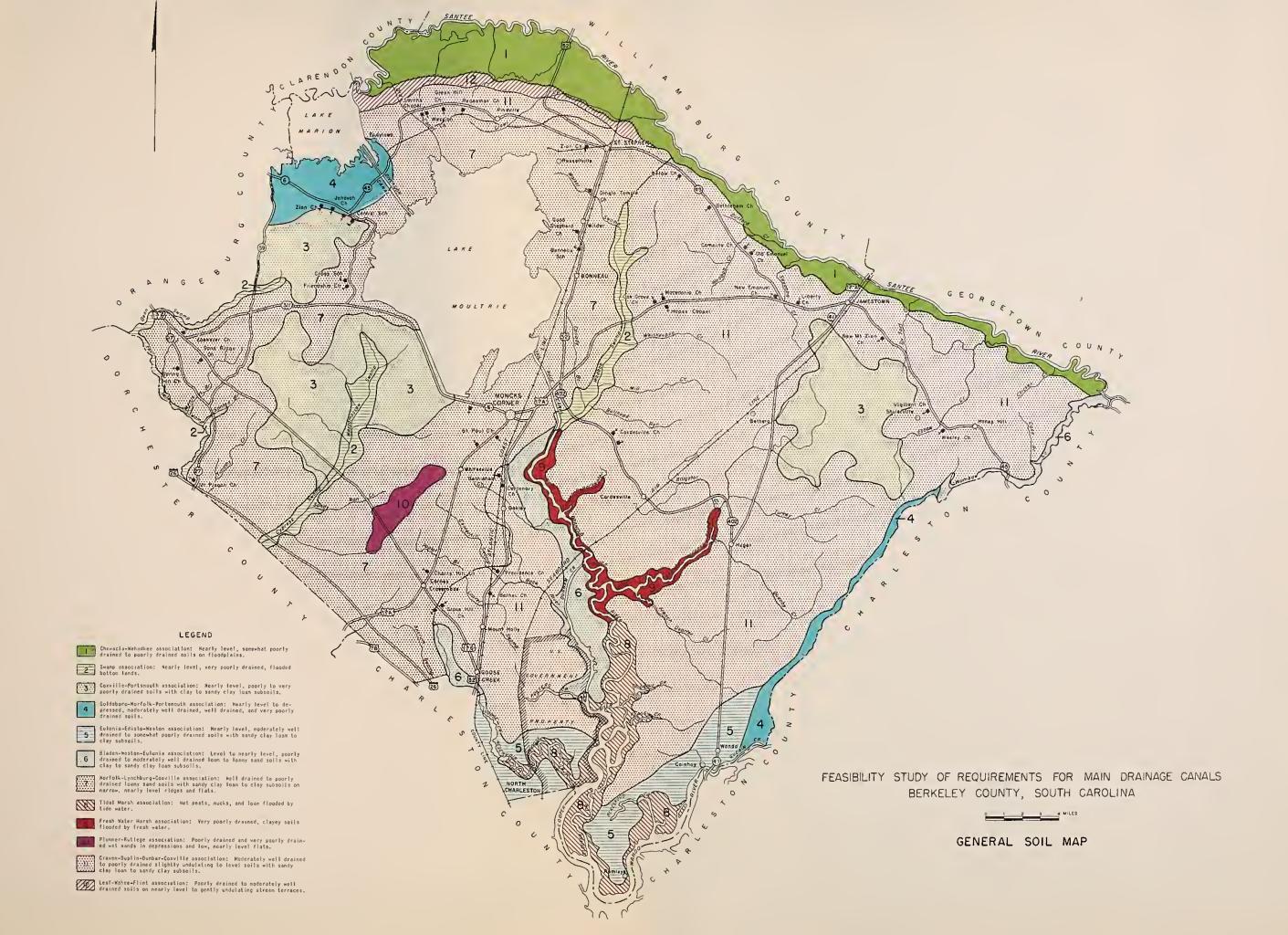
Area 13

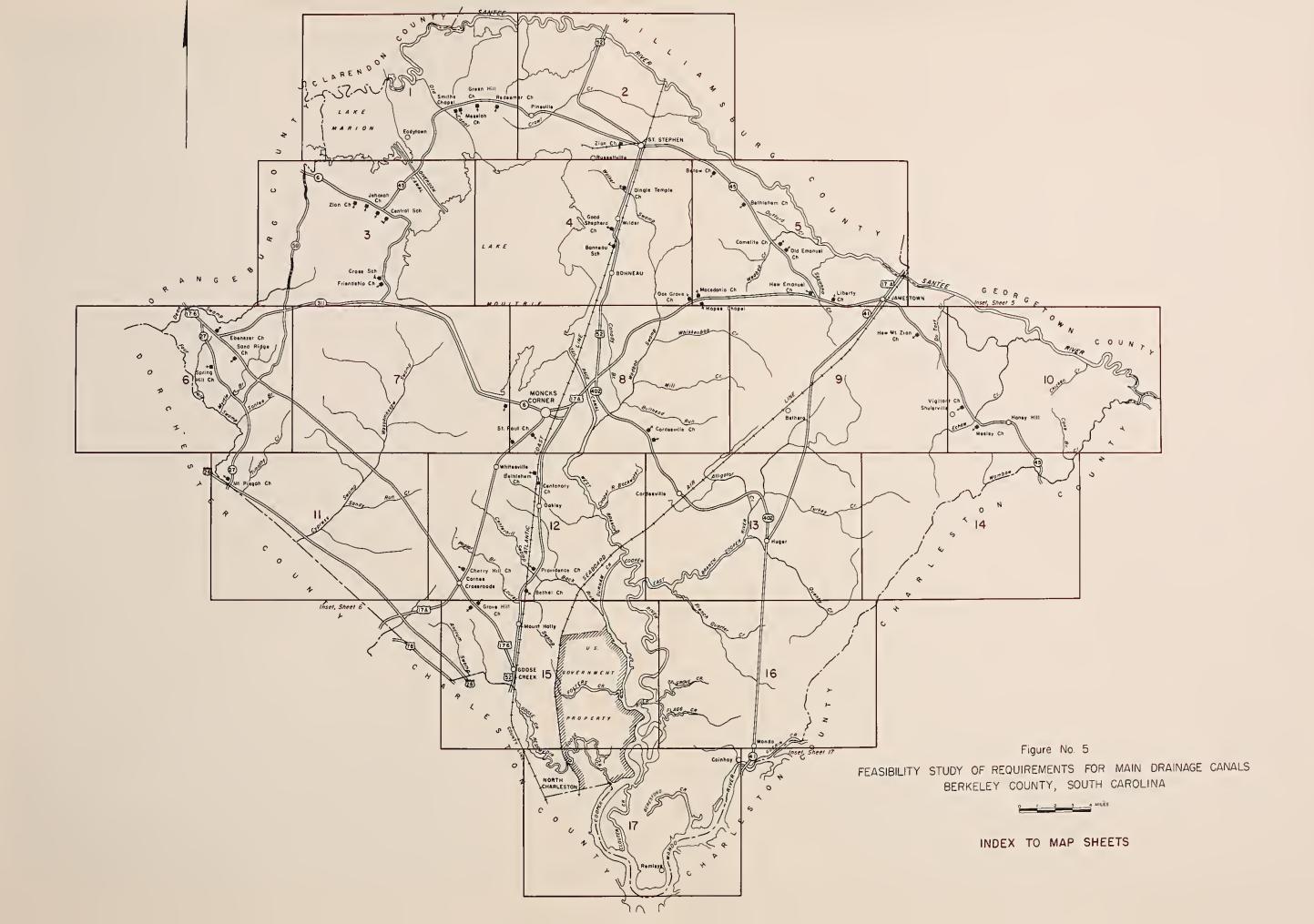
ESTIMATED COST 4609.00 12,933.00 4527.00 3304.00 4519,00 4251.00 8852,00 2040.00 8505.00 7248.00 3016.00 2537.00 Sheet 4 of Dollars (13) TOTAL BRIDGES - NEW Length & Size 15' R.C. Br. 15' R.C. Br. Br. Br. Br. CULVERTS & - 48" 50' - 30" 30" - 24" 48" R.C. C.T. I 1 ı ı 30 1 304 40 4 15' 304 151 LOWERING Length & Size CULVERTS 36" =ŀ 1 1 1 1 | | | 1 1 1 1 1 1 1 1 1 1 1 RT. OF WAY REQUIRED WIDTH Ft. (10) 38 38 38 30 38 38 88 38 38 38 38 38 38 38 38 38 38 38 38 38 33 OF WAY CLEARING 1.1 2.5 , i i i i i i i (6) 6.4 4.8 5.4 2.9 7.5 2.4 3.3 1.2 2.7 0 0 0 4.6 0 0 0 0 Ac. 2.0 RT. EXCAVATION 15,096 2368 17,908 Cu. Yds. 3404 13,764 12,876 9620 9620 10,952 10,952 5920 3404 3996 2220 4736 3108 12,136 7104 5328 5476 5772 5772 9176 4292 6512 2960 19,240 6364 5032 14,652 16,724 6364 (8) AVERAGE ۇپ سىدا DEP TH (2) വവ 10 10 n 20 20 01 01 01 20 20 20 n n 20 10 20 20 വവവ 10 10 CHANNEL DIMENSIONS BO TTOM WIDTH . ئپ (9) ത ന ന m n ന ന ന **.** m m m ന ന (7) m ന ന ന m m m m n WIDTH Ft. (5) T0 P 13 13 13 13 13 13 13 13 13 13 13 13 133 133 DISCHARGE c. f. s. (4) 4 10 00 4 C1 T T 400 0 10 4 4 000-4 0 6 6 12 WATERSHED 100 44 948 224 340 260 584 84 104 316 52 64 660 624 792 52 536 876 208 428 384 484 72 248 224 Ac. (3) 3700 2300 1500 9300 8700 7400 4000 2300 1500 6200 6600 4300 6500 6500 2700 0066 4500 2100 1600 8200 4800 3900 2900 3400 10,200 11,300 LENGTH Ft. (2) M-27 Total-27 L-1Total-29 Total-23 M-31 Total-31 Total-19 Total-26 Total-28 Total -21 Total-22 Total-30 Total-24 Potal-25 M-23 M-23 L-1 M-25 M-25 L-1 M-26 M-26 M-28 M-28 M-30 M-21 M-24 M-24 M-29 M-22 CANAL No. L-1 L-2 L-1

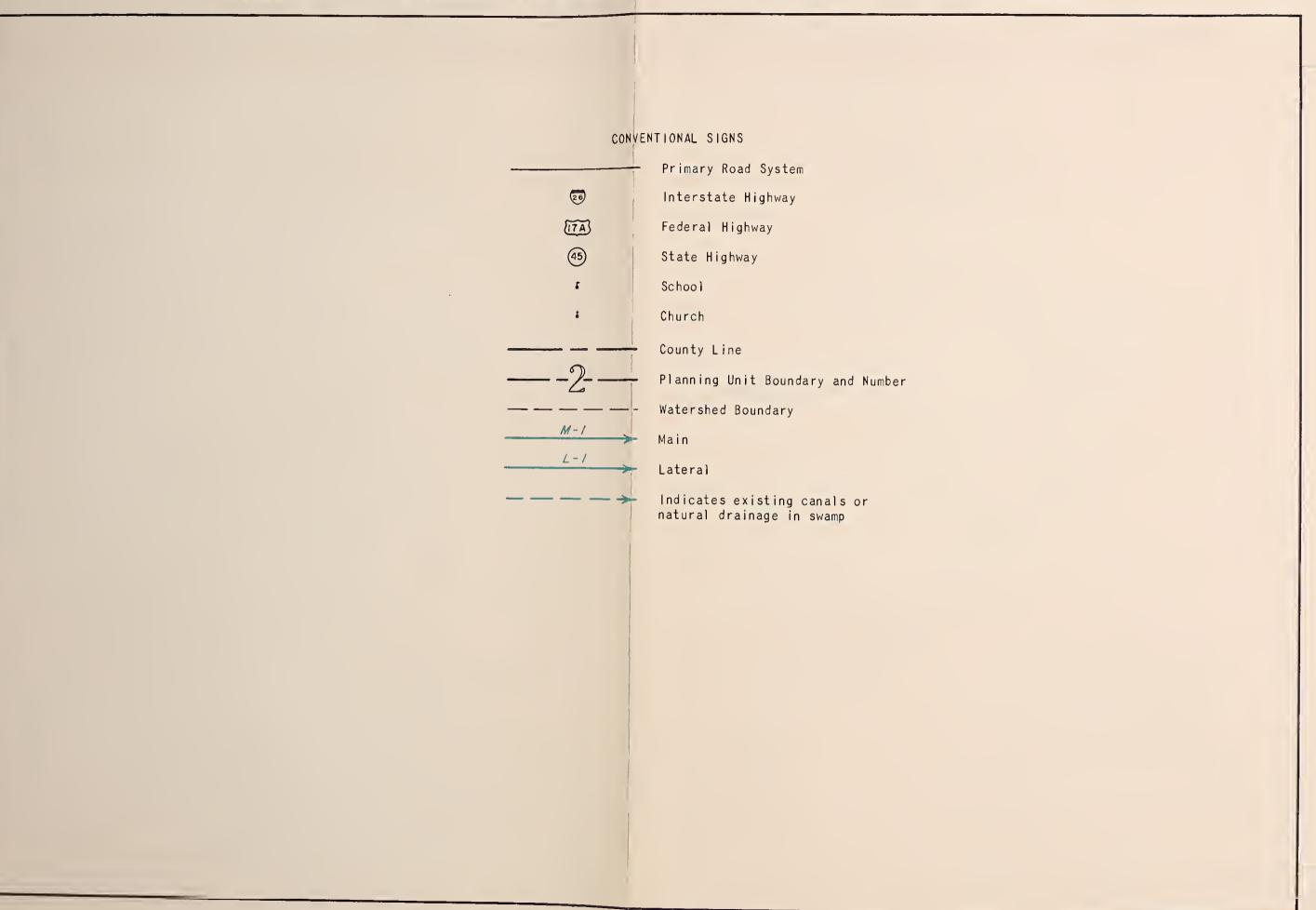
Area 13 - Pompion Chapel - Charity Church - Wando - Daniel's Island

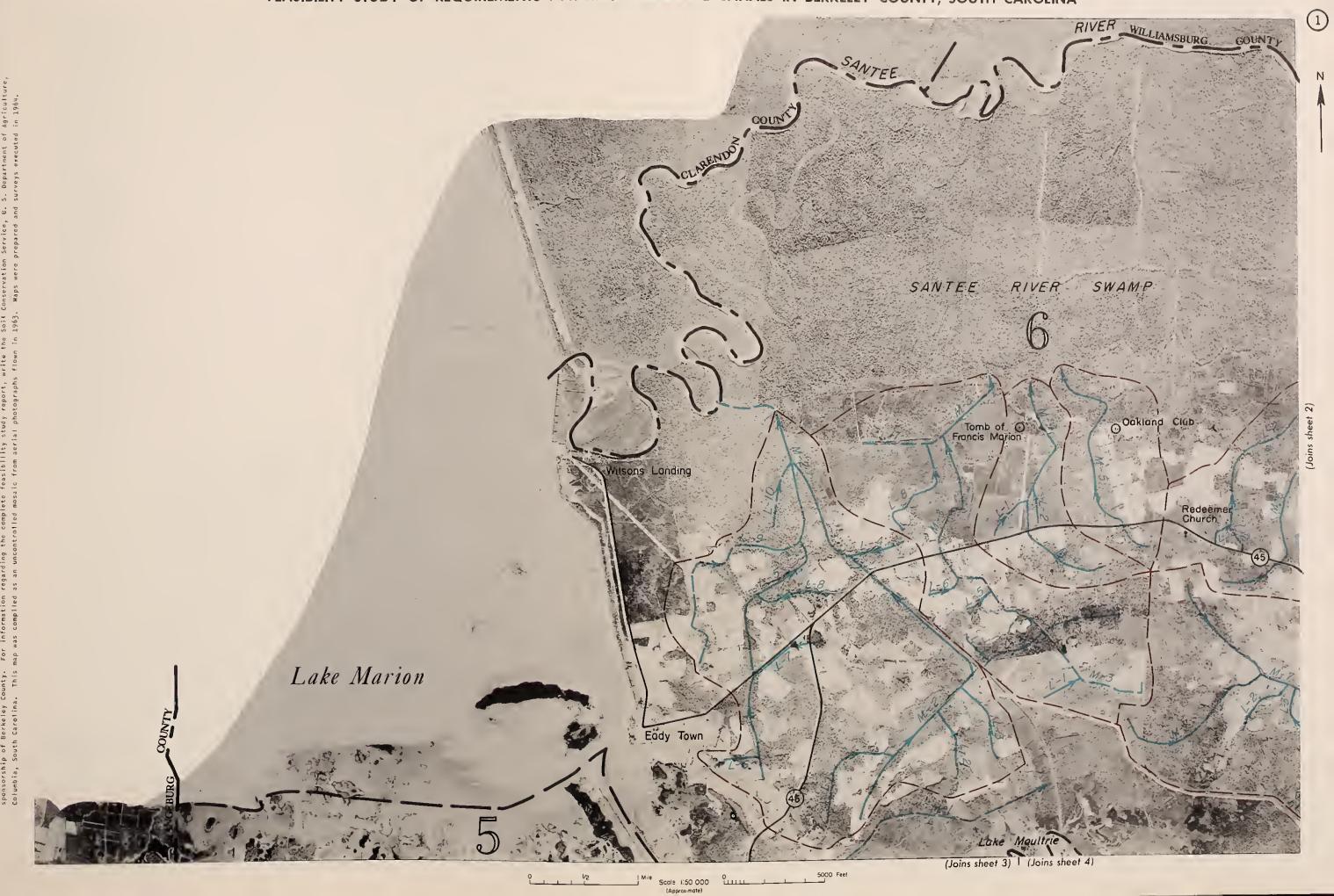
Sheet 5 of 5	ESTIMATED COST	Dollars (13)	(3200.00	5874.00		6240.00	314,504.00	
	CULVERTS & BRIDGES - NEW	Length & Size (12)	20' - 18"		30 30.	30' - 18"			
	CULVERTS	Length & Size (11)			1 t	-			
uo Dann	RT. OF WAY	Ft. (10)	38 38 38	38	38 86	38			
ICH - Wall	WAY	Ac. (9)	1.5	1.1	2.1	2.4	2.4	435.3	
Area 13 - I ompion Chaper - Charity Church - Wallor - Daniel S	EXCAVATION	Cu. Yds. (8)	4144 1036 3848	5772	2308 4292 12,432	10,952	10,952	934,663	
O CHAPTER CO	AVERAGE DEP TH	Ft. (7)	2222	IO H	വര	2			
MEI DIMEN		Ft. (6)		en c	ာ က	က			
CHANNEL	TOP	Ft. (5)	13 13	13	13 13	13			
Alfa 10	DISCHARGE	c. f. s. (4)	11 20 11	13	11	23			
	WATERSHED	Ac. (3)	120 252 112	144	120	296			
	LEN GTH	Ft. (2)	2800	3900	1600 2900 8400	7400	7400	545,200	
	CANAL	No.	M-32 M-32 L-1	Total-32 M-33	M-33 L-1 Total-33	M-34	Total-34	Area 13 Grand Total	

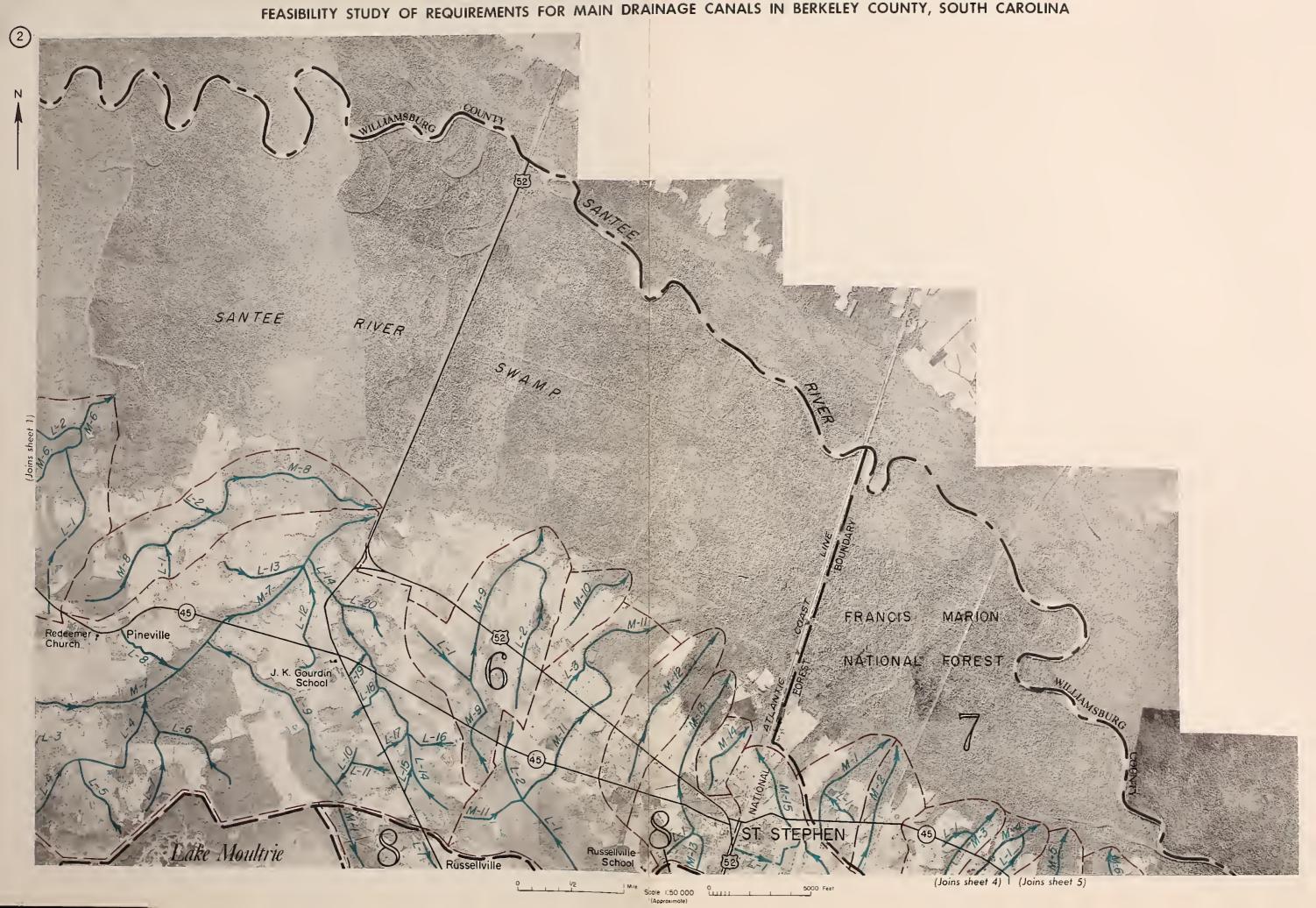


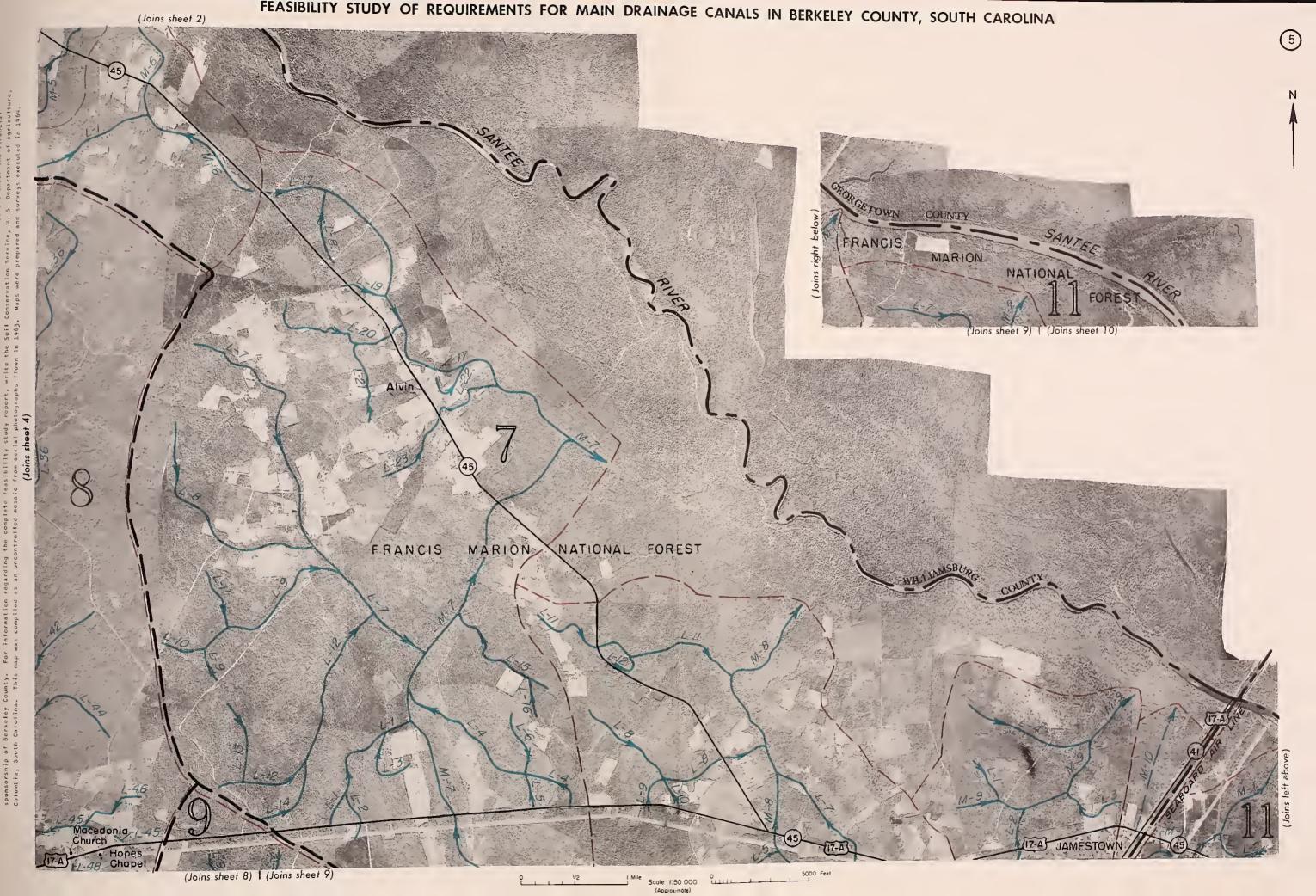


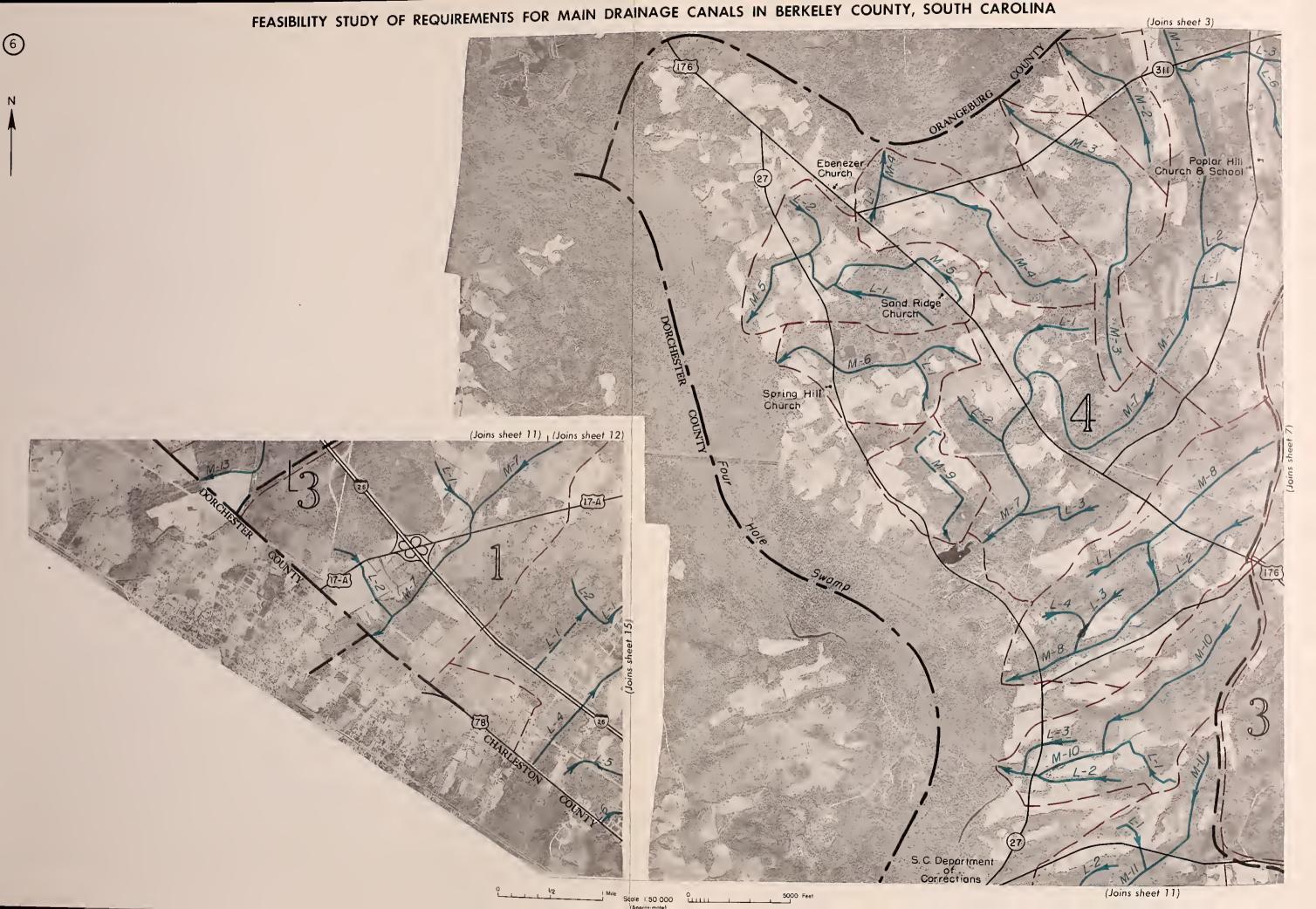




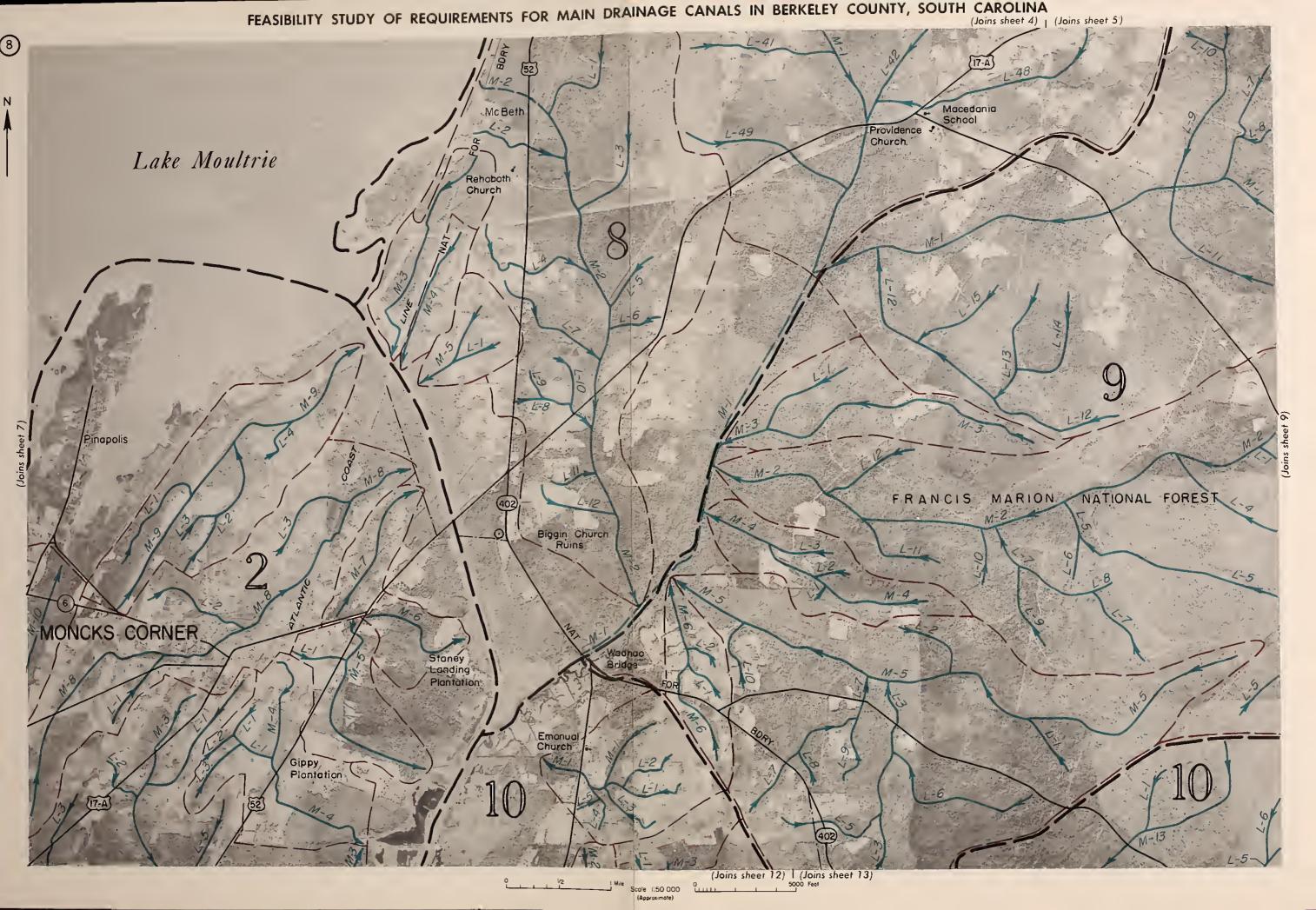


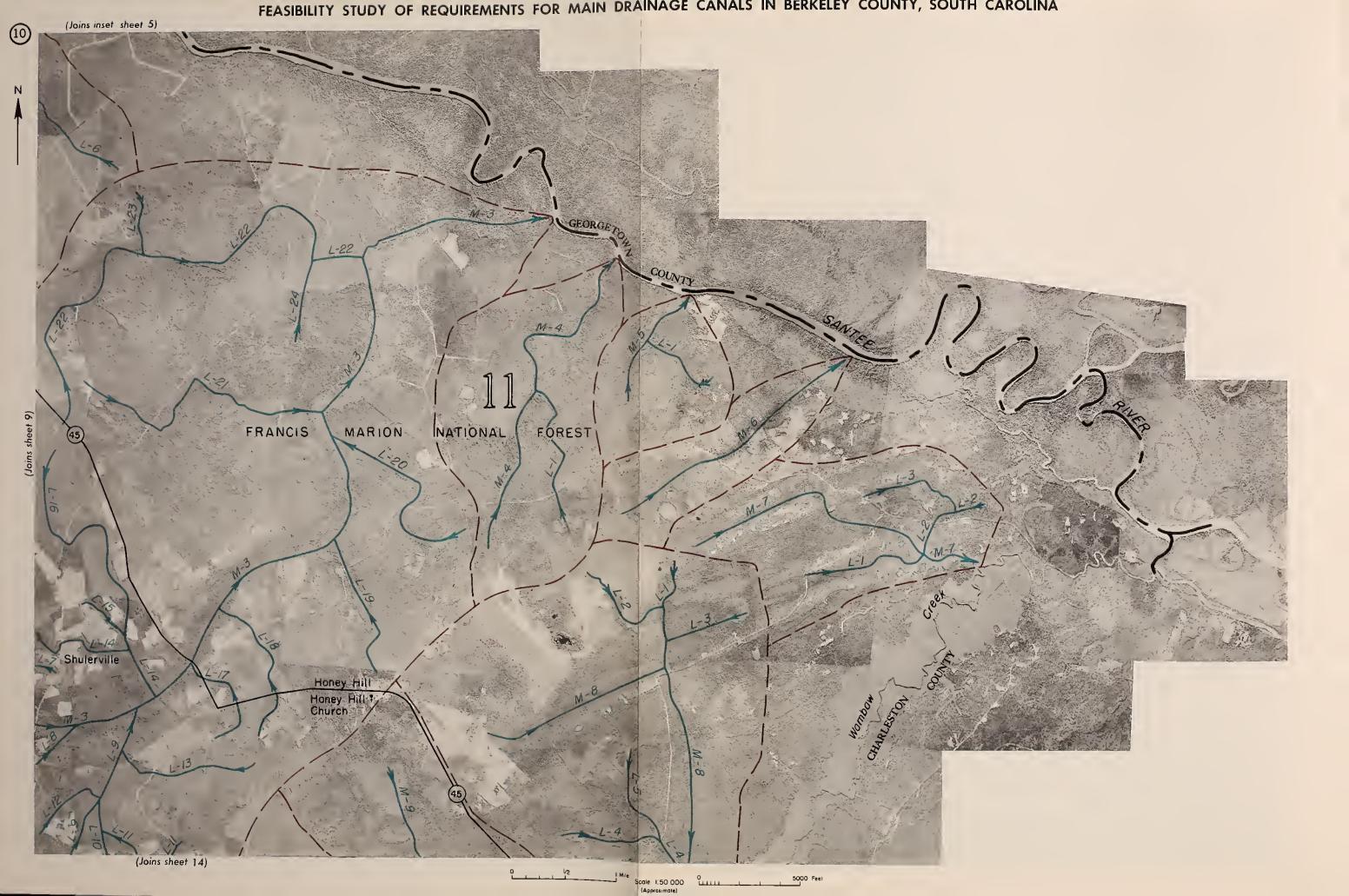


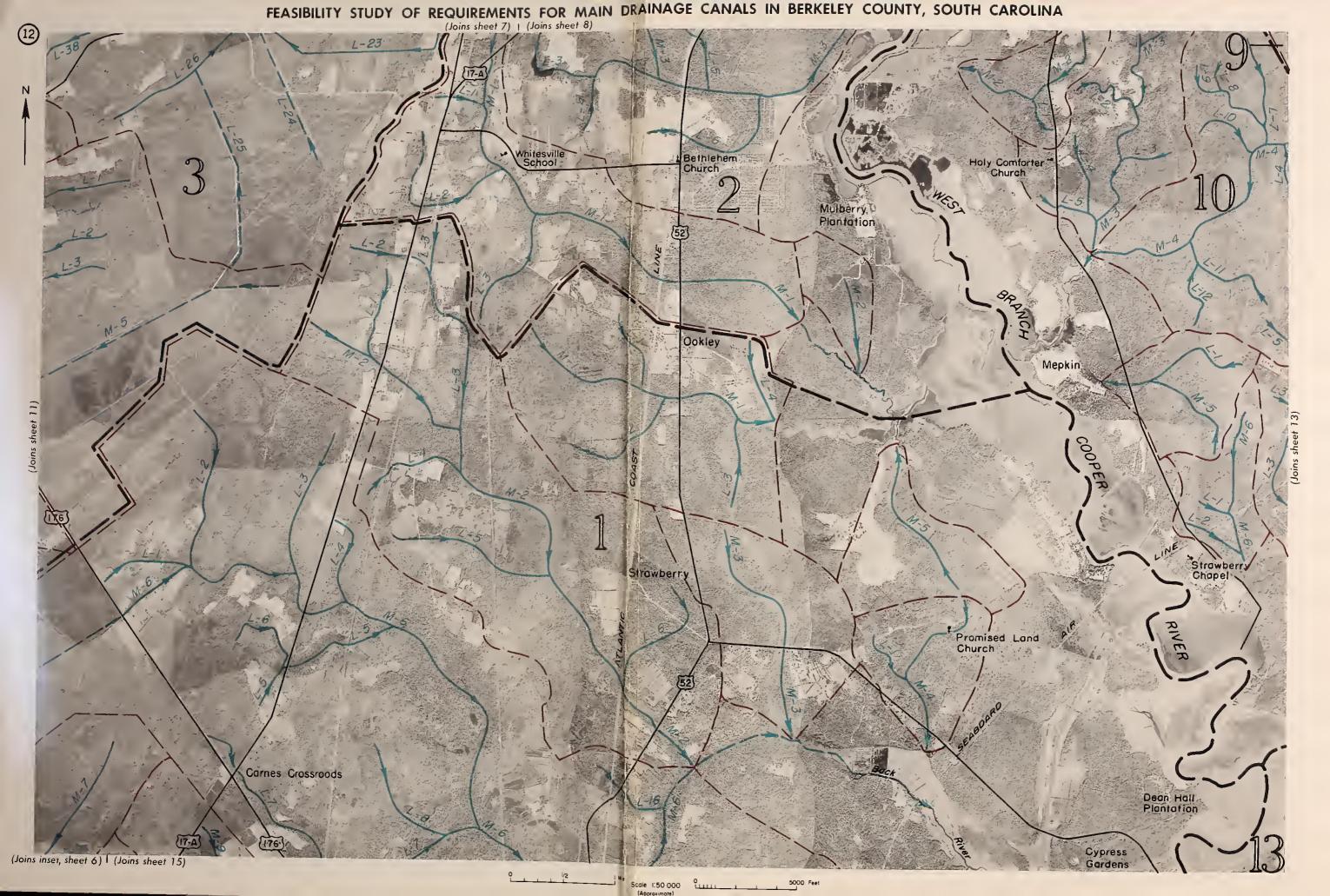




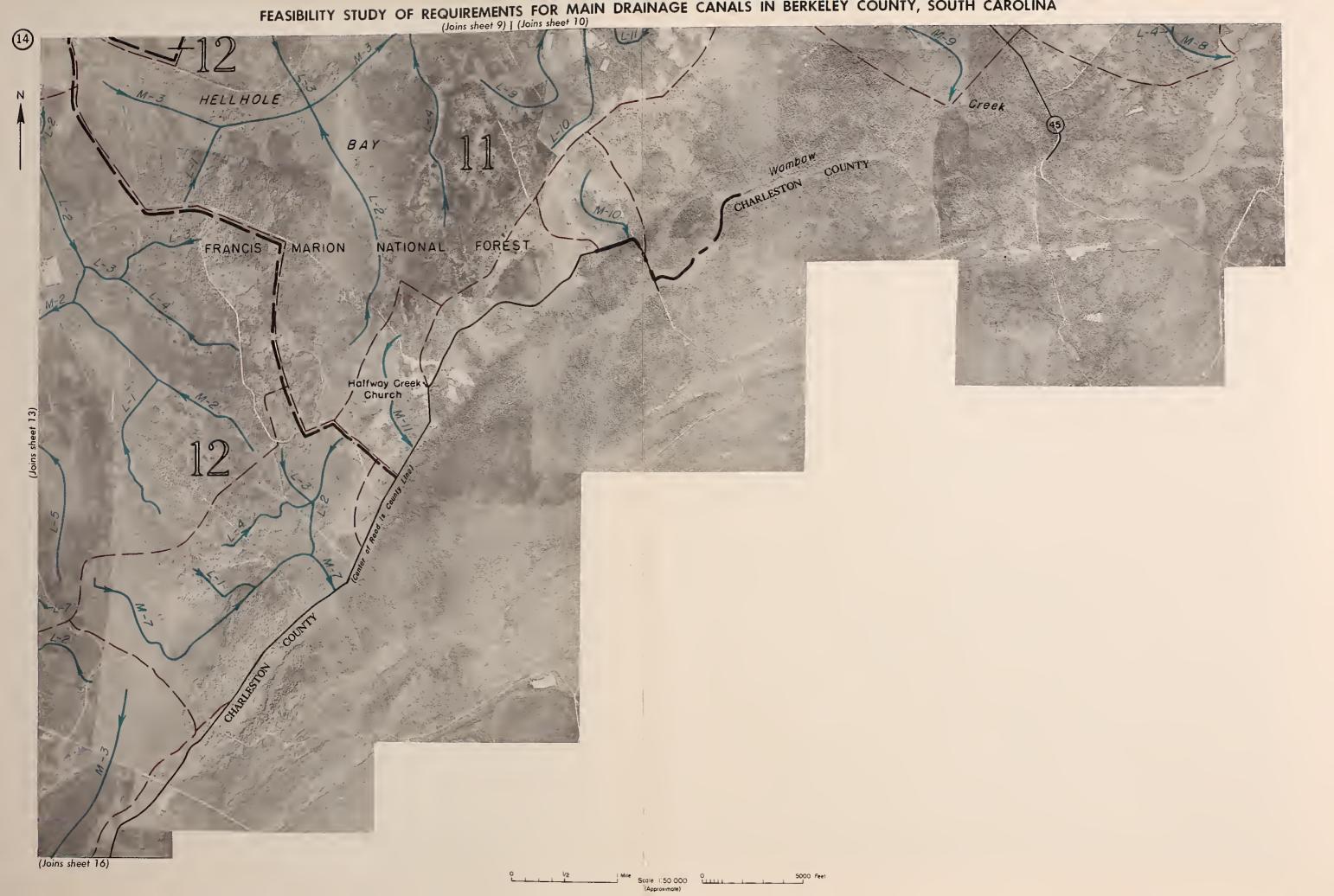


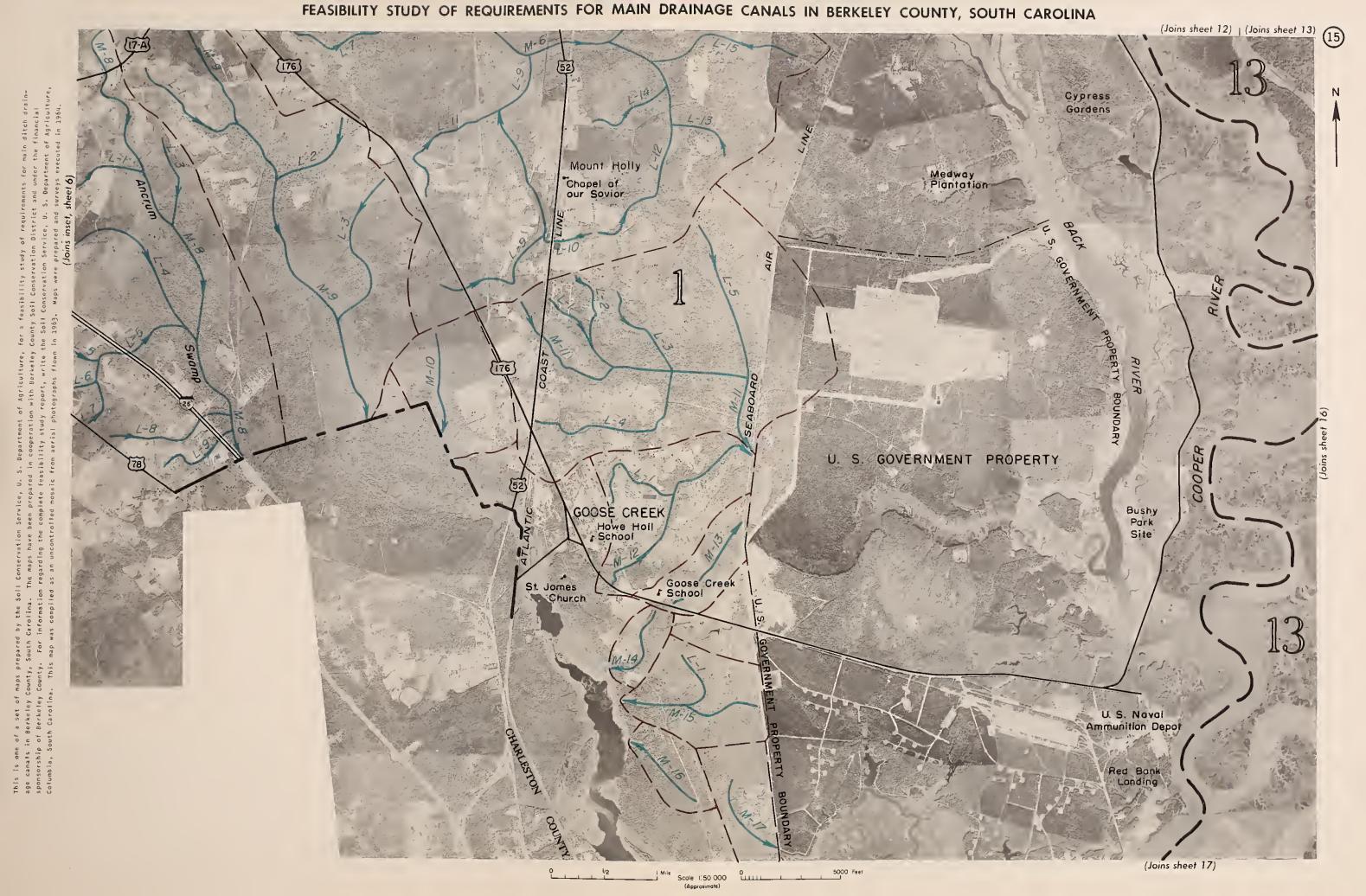


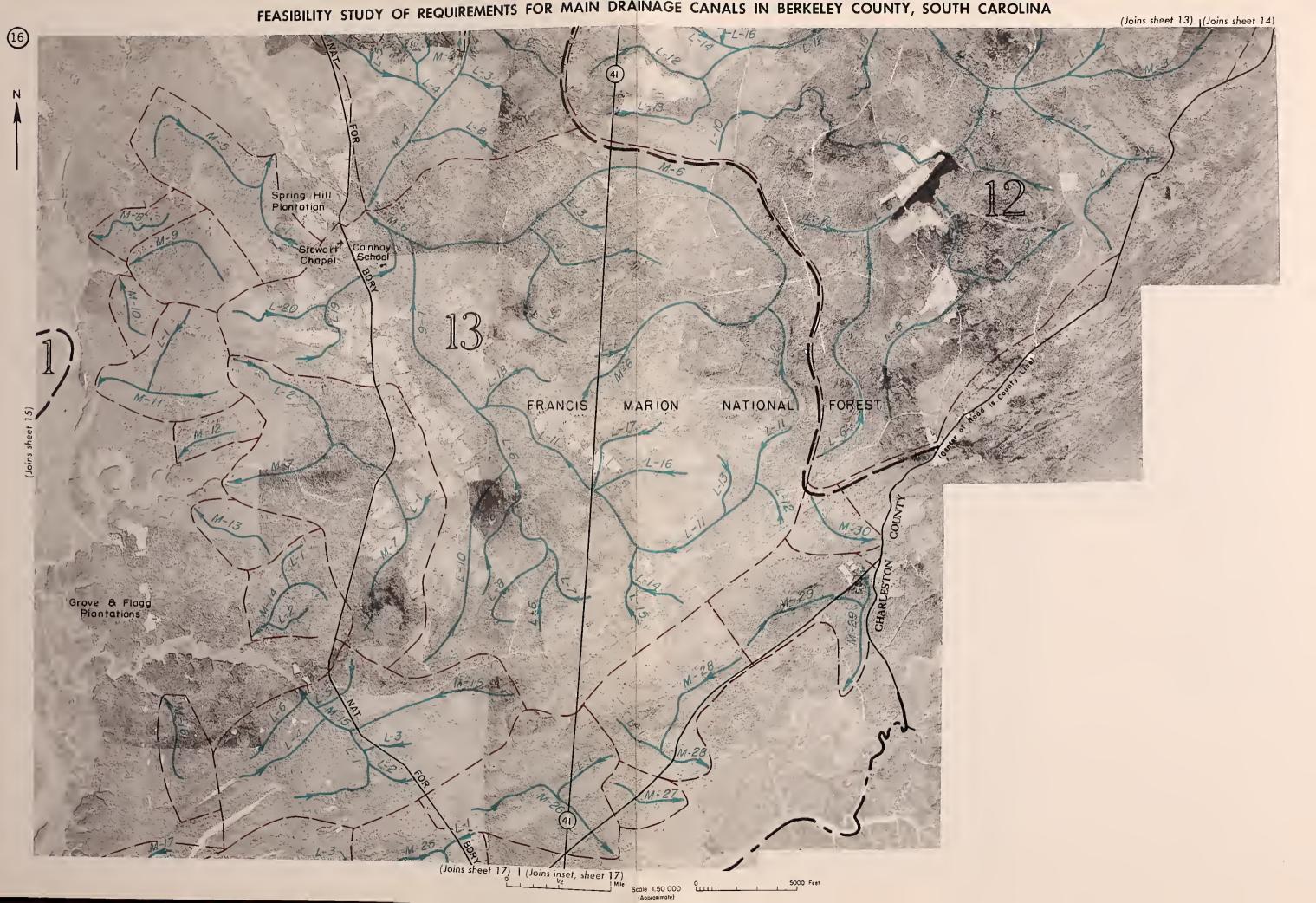




Scale 1:50 000 ULIIII...











R000C 771774

